FEB. '22

STRUCTURAL DESIGN REPORT

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Contents

INTRODUCTION

The designs presented in this report consists of beams, columns, slabs and footings of a three-storey reinforced concrete structure. The various components are modelled and analysed with PROTASTRUCTURE and RC CONCRETE EXCEL SPREADSHEET. The components are designed to resist lateral loads, as well as withstand loads imposed at various periods of its usability. Lateral loads emanating from seismic concerns have also been included in the design similarly. Wind loading was factored in the design.

The components were modelled into various sections using concrete material and maintaining their heights to analyse the effect of wind and earthquake on them.

Material strengths and properties

Concrete grade, *fcu*: 30N/mm^2

Unit Weight of Concrete: 24kN/m^3

Steel grade, *fy*: 460N/mm^2

Minimum bar diameter: 12mm

Structural Detailing and supervision

The drawings accompanying this report are intended to guide the construction process. The Structural Engineer expects that supervision is carried out by suitably qualified professional and may consult the structural designer where need be.

Design Codes and Design Guides

The following codes guided the selection of loads, the analysis and design:

1. The load selection and computation was guided by BS 6399 Part1:1996 *Code of practice for Dead and Imposed load*
2. Seismic load generation was based on Uniform Building Code, UBC 1997
3. Concrete Design was based on BS 8110:1-3

Load Cases and Combinations

34.5 Primary Load Cases

Six (6) primary load cases were analysed:

Earthquake loads in two principal direction

Ground acceleration of 0.2g was used to generate the seismic loads on the foundation structure in the X, and Z directions

Dead loads

The total Dead Loads approximate to **7.1kN/m^2** was applied to the suspended floors(First and Second Floors) and **2.5kN/m^2** at the roof.

Imposed loads

An imposed load applied range from **1.5kN/m^2 – 4.8kN/m^2** depending on the use of floor and guided by BS 6399 Part1:1996 *Code of practice for Dead and Imposed load.*

Load combinations

A total of Twenty-six (25) load combinations were considered based on the recommendations in the Civil Technical Specification and UBC 1997.

Implementing software

The structural frames were modelled in 3D for analysis and design. Modelling, analysis and design were implemented using PROTASTRUCTURE, a popular Structural Engineering Software.

The foundations reactions were transferred to PROTASTRUCTURE.

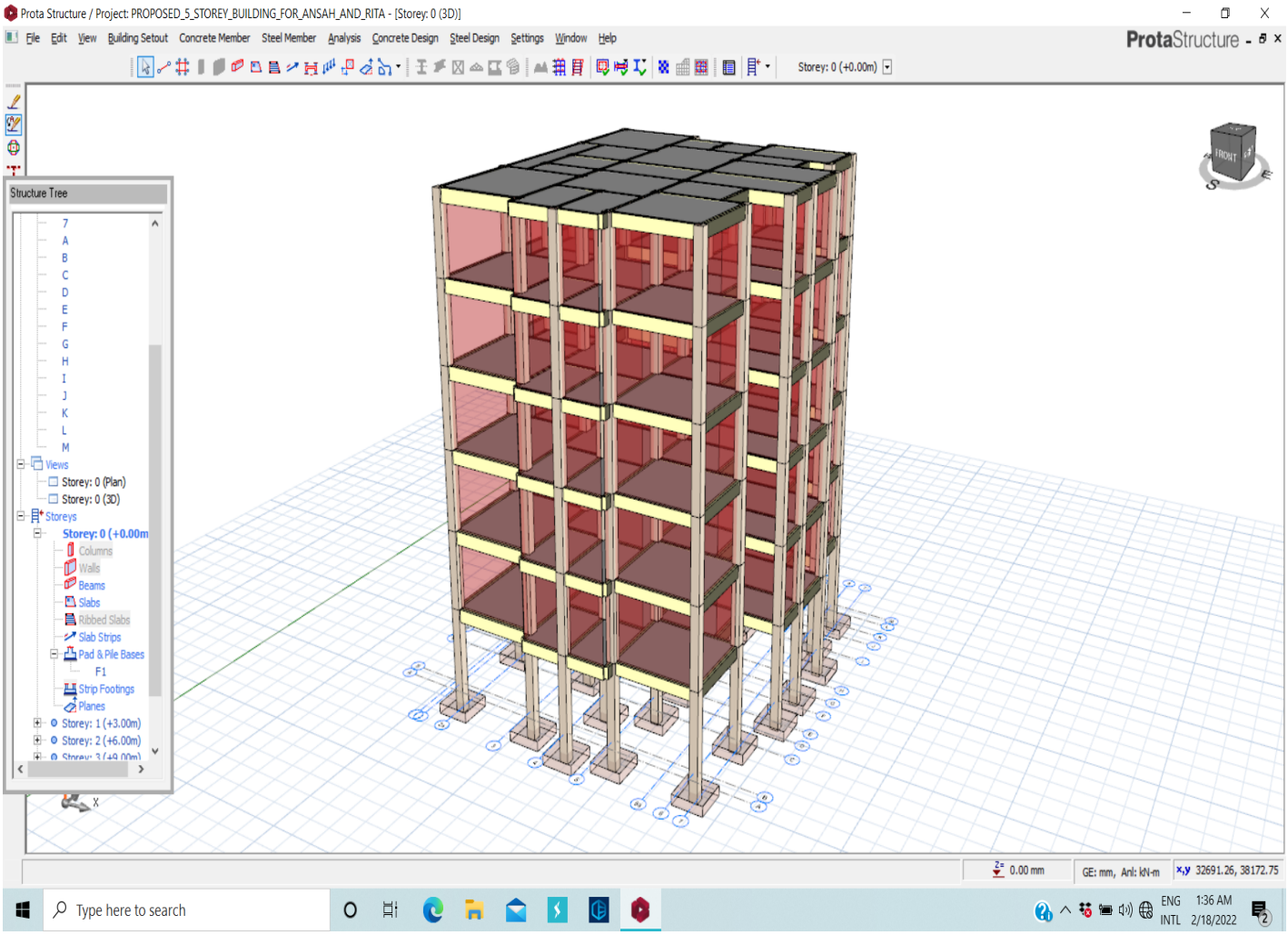
The Report

Modelling in 3D gives a better and economic analysis results because it is close to the actual construction. However, it generates a rather large volume of information that runs into thousands of pages, thus discouraging a printout of the entire input and output files. This report is thus an extract of the calculation processes and results for the analysis and design of the three storey reinforced concrete structure. The design contained in this report assumes a three-storey reinforced concrete building supported on isolated footings. Reactions calculated at these supports are designed for at the foundation.

February, 2022

APPENDIX

STRUCTURAL MODEL



SLAB

Slab Reinforcement Design

```
LEGEND:
    d/h    = Slab Effective/Total Depths (d=h-cover)
    g/q    = Dead/Live Loads (not factored)
    L1     = Width of the Slab Along the Strip Direction
    L2     = Width of the Slab Perpendicular to the Strip Direction
    C      = Moment Coefficient M=C p / L^2
    M-span = Ultimate Span Moment
    M-sup  = Ultimate Support Moment
    Mc     = Balanced Support Moment
    As     = Steel Area (Required/Supplied)


Slab Strip: X1 -- Storey: 1
---------------------------
Materials: C25/30 / Grade 250 (Plain)
```

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
1S1	1	5.600	4025.00	0.0314	0.0242	322.78/287.23 (X)	
	121/150	3.000	4050.00	5.8	4.5		StrBot: R8-175(B1)
			Support Mc = 5.1	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
1S8	1	5.600	2675.00	0.0348	0.0266	322.78/287.23 (X)	
	121/150	3.000	2846.40	2.9	2.2		StrBot: R8-175(B1)
			Support Mc = 2.9	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
1S9	1	5.600	2150.00	0.0470	0.0355	322.78/287.23 (X)	
	121/150	3.000	2846.40	2.5	1.9		StrBot: R8-175(B1)
			Support Mc = 2.2	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
1S10	1	5.600	1475.00	0.0619	0.0469	322.78/287.23 (X)	
	121/150	3.000	2846.40	1.5	1.2		StrBot: R8-175(B1)
			Support Mc = 5.4	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
1S11	1	5.600	4650.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	4050.00	6.0	4.5		StrBot: R8-200(B2)
			Support Mc = 0.7	Support As = 301.43/0.00			

```
Slab Strip: X2 -- Storey: 1
---------------------------
Materials: C25/30 / Grade 250 (Plain)
```

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.0		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
1S7	1 113/150	5.600 3.000	6300.00 1203.60	0.0000 0.0	0.0000 0.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.0		Support As = 301.43/0.00		

Slab Strip: X3 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
1S2	1 121/150	5.600 3.000	1950.00 2100.00	0.0356 1.6	0.0271 1.2	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 5.5		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
1S6	1 121/150	5.600 3.000	4150.00 4300.00	0.0332 6.6	0.0254 5.0	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 0.8		Support As = 322.78/0.00		

Slab Strip: X4 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
1S12	1 121/150	5.600 3.000	3425.00 4300.00	0.0442 6.0	0.0337 4.5	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 4.6		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
1S14	1 113/150	5.600 3.000	3300.00 2200.00	0.0320 1.8	0.0240 1.3	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2		Support As = 301.43/0.00		

Slab Strip: X5 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S3	1	5.600	1950.00	0.0325	0.0250	322.78/287.23 (X)	
	121/150	3.000	2000.00	1.4	1.1		StrBot: R8-175(B1)
			Support Mc = 5.5			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S6	1	5.600	4150.00	0.0332	0.0254	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.6	5.0		StrBot: R8-175(B1)
			Support Mc = 0.8			Support As = 322.78/0.00	

Slab Strip: X6 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S12	1	5.600	3425.00	0.0442	0.0337	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.0	4.5		StrBot: R8-175(B1)
			Support Mc = 4.5			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S13	1	5.600	3300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2100.00	1.6	1.2		StrBot: R8-200(B2)
			Support Mc = 0.2			Support As = 301.43/0.00	

Slab Strip: X7 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.5			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S4	1	5.600	2550.00	0.0551	0.0414	322.78/287.23 (X)	
	121/150	3.000	4050.00	4.1	3.1		StrBot: R8-175(B1)
			Support Mc = 3.5			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)

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 1S5             1      5.600    1475.00     0.0630     0.0630 322.78/287.23 (X)
          121/150       3.000    2989.50        1.6        1.6                    StrBot: R8-175(B1)

                                  Support Mc = 4.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S17            1      5.600    2675.00     0.0606     0.0456 322.78/287.23 (X)
          121/150       3.000    4950.00        5.0        3.8                    StrBot: R8-175(B1)

                                  Support Mc = 4.5Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S16            1      5.600    2150.00     0.0630     0.0630 322.78/287.23 (X)
          121/150       3.000    4950.00        3.3        3.3                    StrBot: R8-175(B1)

                                  Support Mc = 3.3Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S18            1      5.600    2375.00     0.0443     0.0338 322.78/287.23 (X)
          121/150       3.000    2989.50        2.9        2.2                    StrBot: R8-175(B1)

                                  Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S15            1      5.600    3750.00     0.0358     0.0272 322.78/287.23 (X)
          121/150       3.000    4050.00        5.8        4.4                    StrBot: R8-175(B1)

                                  Support Mc = 0.7 Support As = 322.78/0.00


Slab Strip: X8 -- Storey: 1
---------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span           As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)       (mm2)

                                  Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S7             1      5.600    1203.60     0.0860     0.0860 322.78/287.23 (X)
          121/150       3.000    6300.00        1.4        1.4                    StrBot: R8-175(B1)

                                  Support Mc = 2.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S8             1      5.600    2846.40     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    2675.00        2.6        2.0                    StrBot: R8-200(B2)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S6             1      5.600    4300.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    4150.00        6.3        4.8                    StrBot: R8-200(B2)

                                  Support Mc = 5.0Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S17            1      5.600    4950.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    2675.00        2.6        2.0                    StrBot: R8-200(B2)
```

Support Mc = 0.3 Support As = 301.43/0.00

Slab Strip: X9 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S1	1 121/150	5.600 3.000	4050.00 4025.00	0.0320 6.0	0.0240 4.5	322.78/287.23 (X)	StrBot: R8-175(B1)
				Support Mc = 4.9		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S2	1 113/150	5.600 3.000	2200.00 1950.00	0.0320 1.4	0.0240 1.0	301.43/251.33 (X)	StrBot: R8-200(B2)
				Support Mc = 1.4		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S3	1 113/150	5.600 3.000	2100.00 1950.00	0.0320 1.4	0.0240 1.0	301.43/251.33 (X)	StrBot: R8-200(B2)
				Support Mc = 2.2		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S4	1 113/150	5.600 3.000	4050.00 2550.00	0.0320 2.4	0.0240 1.8	301.43/251.33 (X)	StrBot: R8-200(B2)
				Support Mc = 0.3		Support As = 301.43/0.00	

Slab Strip: X10 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S6	1 113/150	5.600 3.000	4300.00 4150.00	0.0320 6.3	0.0240 4.8	301.43/251.33 (X)	StrBot: R8-200(B2)
				Support Mc = 6.3		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S5	1 113/150	5.600 3.000	2989.50 1475.00	0.0000 0.0	0.0000 0.0	301.43/251.33 (X)	StrBot: R8-200(B2)
				Support Mc = 0.0		Support As = 301.43/0.00	

Slab Strip: X11 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.2		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S7	1	5.600	1203.60	0.1000	0.0860	322.78/287.23 (X)	
	121/150	3.000	6300.00	1.7	1.4		StrBot: R8-175(B1)
				Support Mc = 1.7		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S9	1	5.600	2846.40	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2150.00	1.7	1.3		StrBot: R8-200(B2)
				Support Mc = 0.2		Support As = 301.43/0.00	

Slab Strip: X12 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.0		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S16	1	5.600	4950.00	0.0000	0.0000	301.43/251.33 (X)	
	113/150	3.000	2150.00	0.0	0.0		StrBot: R8-200(B2)
				Support Mc = 0.0		Support As = 301.43/0.00	

Slab Strip: X13 -- Storey: 1

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.8		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
1S11	1	5.600	4050.00	0.0394	0.0299	322.78/287.23 (X)	
	121/150	3.000	4650.00	7.4	5.6		StrBot: R8-175(B1)
				Support Mc = 6.3		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
1S12	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	3325.00	4.1	3.1		StrBot: R8-200(B2)

```
                                  Support Mc = 3.5Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S18            1      5.600    2989.50     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    2375.00        2.1        1.6                   StrBot: R8-200(B2)

                                  Support Mc = 0.2 Support As = 301.43/0.00


Slab Strip: X14 -- Storey: 1
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S7             1      5.600    1203.60     0.0860     0.0860 322.78/287.23 (X)
           121/150      3.000    6300.00        1.4        1.4                   StrBot: R8-175(B1)

                                  Support Mc = 1.1Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S10            1      5.600    2846.40     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    1475.00        0.8        0.6                   StrBot: R8-200(B2)

                                  Support Mc = 3.2Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S12            1      5.600    4300.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    3325.00        4.1        3.1                   StrBot: R8-200(B2)

                                  Support Mc = 3.5Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S18            1      5.600    2989.50     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    2375.00        2.1        1.6                   StrBot: R8-200(B2)

                                  Support Mc = 0.2 Support As = 301.43/0.00


Slab Strip: X15 -- Storey: 1
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.8Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S11            1      5.600    4050.00     0.0394     0.0299 322.78/287.23 (X)
           121/150      3.000    4650.00        7.4        5.6                   StrBot: R8-175(B1)

                                  Support Mc = 6.4Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)
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 1S14            1      5.600    2200.00     0.0530     0.0400 322.78/287.23 (X)
          121/150       3.000    3300.00        2.9        2.2                    StrBot: R8-175(B1)

                                  Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 1S13            1      5.600    2100.00     0.0547     0.0411 322.78/287.23 (X)
          121/150       3.000    3300.00        2.8        2.1                    StrBot: R8-175(B1)

                                  Support Mc = 4.6Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 1S15            1      5.600    4050.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    3750.00        5.2        3.9                    StrBot: R8-200(B2)

                                  Support Mc = 0.6 Support As = 301.43/0.00


Slab Strip: X15 -- Storey: 2
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span            As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S1             1      5.600    4025.00     0.0314     0.0242 322.78/287.23 (X)
          121/150       3.000    4050.00        5.8        4.5                    StrBot: R8-175(B1)

                                  Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S8             1      5.600    2675.00     0.0348     0.0266 322.78/287.23 (X)
          121/150       3.000    2846.40        2.9        2.2                    StrBot: R8-175(B1)

                                  Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S9             1      5.600    2150.00     0.0470     0.0355 322.78/287.23 (X)
          121/150       3.000    2846.40        2.5        1.9                    StrBot: R8-175(B1)

                                  Support Mc = 2.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S10            1      5.600    1475.00     0.0619     0.0469 322.78/287.23 (X)
          121/150       3.000    2846.40        1.5        1.2                    StrBot: R8-175(B1)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 2S11            1      5.600    4650.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    4050.00        6.0        4.5                    StrBot: R8-200(B2)

                                  Support Mc = 0.7 Support As = 301.43/0.00


Slab Strip: X16 -- Storey: 2
```

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.0		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
2S7	1 113/150	5.600 3.000	6300.00 1203.60	0.0000 0.0	0.0000 0.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.0		Support As = 301.43/0.00		

Slab Strip: X17 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
2S2	1 121/150	5.600 3.000	1950.00 2100.00	0.0356 1.6	0.0271 1.2	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 5.5		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
2S6	1 121/150	5.600 3.000	4150.00 4300.00	0.0332 6.6	0.0254 5.0	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 0.8		Support As = 322.78/0.00		

Slab Strip: X18 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
2S12	1 121/150	5.600 3.000	3425.00 4300.00	0.0442 6.0	0.0337 4.5	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 4.6		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
2S14	1 113/150	5.600 3.000	3300.00 2200.00	0.0320 1.8	0.0240 1.3	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2		Support As = 301.43/0.00		

Slab Strip: X19 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
2S13	1 113/150	5.600 3.000	3300.00 2100.00	0.0000 0.2	0.0240 1.2	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2	Support As = 301.43/0.00			

Slab Strip: X20 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
2S3	1 121/150	5.600 3.000	1950.00 2000.00	0.0000 0.2	0.0250 1.1	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 0.2	Support As = 322.78/0.00			

Slab Strip: X21 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.5	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
2S4	1 121/150	5.600 3.000	2550.00 4050.00	0.0551 4.1	0.0414 3.1	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 3.5	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
2S5	1 121/150	5.600 3.000	1475.00 2989.50	0.0630 1.6	0.0630 1.6	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 4.2	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
2S17	1	5.600	2675.00	0.0606	0.0456	322.78/287.23 (X)	

```
          121/150      3.000    4950.00        5.0        3.8                    StrBot: R8-175(B1)

                                  Support Mc = 4.5Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S16           1      5.600    2150.00     0.0630     0.0630 322.78/287.23 (X)
          121/150      3.000    4950.00        3.3        3.3                    StrBot: R8-175(B1)

                                  Support Mc = 3.3Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S18           1      5.600    2375.00     0.0443     0.0338 322.78/287.23 (X)
          121/150      3.000    2989.50        2.9        2.2                    StrBot: R8-175(B1)

                                  Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S15           1      5.600    3750.00     0.0358     0.0272 322.78/287.23 (X)
          121/150      3.000    4050.00        5.8        4.4                    StrBot: R8-175(B1)

                                  Support Mc = 0.7 Support As = 322.78/0.00


Slab Strip: X22 -- Storey: 2
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span            As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 2S1            1      5.600    4050.00     0.0320     0.0240 322.78/287.23 (X)
          121/150      3.000    4025.00        6.0        4.5                    StrBot: R8-175(B1)

                                  Support Mc = 4.9Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 2S2            1      5.600    2200.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    1950.00        1.4        1.0                    StrBot: R8-200(B2)

                                  Support Mc = 1.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 2S3            1      5.600    2100.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    1950.00        1.4        1.0                    StrBot: R8-200(B2)

                                  Support Mc = 2.2Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 2S4            1      5.600    4050.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    2550.00        2.4        1.8                    StrBot: R8-200(B2)

                                  Support Mc = 0.3 Support As = 301.43/0.00


Slab Strip: X23 -- Storey: 2
----------------------------
Materials: C25/30 / Grade 250 (Plain)
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Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
2S6	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	4150.00	6.3	4.8		StrBot: R8-200(B2)
				Support Mc = 6.3		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
2S5	1	5.600	2989.50	0.0000	0.0000	301.43/251.33 (X)	
	113/150	3.000	1475.00	0.0	0.0		StrBot: R8-200(B2)
				Support Mc = 0.0		Support As = 301.43/0.00	

Slab Strip: X24 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.2		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
2S7	1	5.600	1203.60	0.0860	0.0860	322.78/287.23 (X)	
	121/150	3.000	6300.00	1.4	1.4		StrBot: R8-175(B1)
				Support Mc = 2.4		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
2S8	1	5.600	2846.40	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2675.00	2.6	2.0		StrBot: R8-200(B2)
				Support Mc = 5.4		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
2S6	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	4150.00	6.3	4.8		StrBot: R8-200(B2)
				Support Mc = 5.0		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
2S17	1	5.600	4950.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2675.00	2.6	2.0		StrBot: R8-200(B2)
				Support Mc = 0.3		Support As = 301.43/0.00	

Slab Strip: X25 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Type	g	L1	C-sup	C-span	As

Slab	d/h (mm)	q (kN/m2)	L2 (mm)	M-sup (kN.m)	M-span (kN.m)	Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
2S7	1	5.600	1203.60	0.1000	0.0860	322.78/287.23 (X)	
	121/150	3.000	6300.00	1.7	1.4		StrBot: R8-175(B1)
			Support Mc = 1.7	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
2S9	1	5.600	2846.40	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2150.00	1.7	1.3		StrBot: R8-200(B2)
			Support Mc = 0.2	Support As = 301.43/0.00			

Slab Strip: X26 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.0	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
2S16	1	5.600	4950.00	0.0000	0.0000	301.43/251.33 (X)	
	113/150	3.000	2150.00	0.0	0.0		StrBot: R8-200(B2)
			Support Mc = 0.0	Support As = 301.43/0.00			

Slab Strip: X27 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.1	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
2S10	1	5.600	2846.40	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	1475.00	0.8	0.6		StrBot: R8-200(B2)
			Support Mc = 3.2	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
2S12	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	3325.00	4.1	3.1		StrBot: R8-200(B2)
			Support Mc = 3.5	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
2S18	1	5.600	2989.50	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2375.00	2.1	1.6		StrBot: R8-200(B2)

Support Mc = 0.2 Support As = 301.43/0.00

Slab Strip: X28 -- Storey: 2

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.8		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
2S11	1 121/150	5.600 3.000	4050.00 4650.00	0.0394 7.4	0.0299 5.6	322.78/287.23 (X)	StrBot: R8-175(B1)
				Support Mc = 6.4		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
2S14	1 121/150	5.600 3.000	2200.00 3300.00	0.0530 2.9	0.0400 2.2	322.78/287.23 (X)	StrBot: R8-175(B1)
				Support Mc = 2.9		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
2S13	1 121/150	5.600 3.000	2100.00 3300.00	0.0547 2.8	0.0411 2.1	322.78/287.23 (X)	StrBot: R8-175(B1)
				Support Mc = 4.6		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
2S15	1 113/150	5.600 3.000	4050.00 3750.00	0.0320 5.2	0.0240 3.9	301.43/251.33 (X)	StrBot: R8-200(B2)
				Support Mc = 0.6		Support As = 301.43/0.00	

Slab Strip: X28 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
3S1	1 121/150	5.600 3.000	4025.00 4050.00	0.0314 5.8	0.0242 4.5	322.78/287.23 (X)	StrBot: R8-175(B1)
				Support Mc = 5.1		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
3S8	1 121/150	5.600 3.000	2675.00 2846.40	0.0348 2.9	0.0266 2.2	322.78/287.23 (X)	StrBot: R8-175(B1)
				Support Mc = 2.9		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)

```
 3S9             1      5.600    2150.00     0.0470     0.0355 322.78/287.23 (X)
           121/150      3.000    2846.40        2.5        1.9                    StrBot: R8-175(B1)

                                  Support Mc = 2.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S10            1      5.600    1475.00     0.0619     0.0469 322.78/287.23 (X)
           121/150      3.000    2846.40        1.5        1.2                    StrBot: R8-175(B1)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 3S11            1      5.600    4650.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    4050.00        6.0        4.5                    StrBot: R8-200(B2)

                                  Support Mc = 0.7 Support As = 301.43/0.00


Slab Strip: X29 -- Storey: 3
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)    (kN/m2)       (mm)    (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.5Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S4             1      5.600    2550.00     0.0551     0.0414 322.78/287.23 (X)
           121/150      3.000    4050.00        4.1        3.1                    StrBot: R8-175(B1)

                                  Support Mc = 3.5Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S5             1      5.600    1475.00     0.0630     0.0630 322.78/287.23 (X)
           121/150      3.000    2989.50        1.6        1.6                    StrBot: R8-175(B1)

                                  Support Mc = 4.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S17            1      5.600    2675.00     0.0606     0.0456 322.78/287.23 (X)
           121/150      3.000    4950.00        5.0        3.8                    StrBot: R8-175(B1)

                                  Support Mc = 4.5Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S16            1      5.600    2150.00     0.0630     0.0630 322.78/287.23 (X)
           121/150      3.000    4950.00        3.3        3.3                    StrBot: R8-175(B1)

                                  Support Mc = 3.3Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S18            1      5.600    2375.00     0.0443     0.0338 322.78/287.23 (X)
           121/150      3.000    2989.50        2.9        2.2                    StrBot: R8-175(B1)

                                  Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 3S15            1      5.600    3750.00     0.0358     0.0272 322.78/287.23 (X)
           121/150      3.000    4050.00        5.8        4.4                    StrBot: R8-175(B1)
```

Support Mc = 0.7 Support As = 322.78/0.00

Slab Strip: X30 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
3S1	1	5.600	4050.00	0.0320	0.0240	322.78/287.23 (X)	
	121/150	3.000	4025.00	6.0	4.5		StrBot: R8-175(B1)
				Support Mc = 4.9		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S2	1	5.600	2200.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	1950.00	1.4	1.0		StrBot: R8-200(B2)
				Support Mc = 1.4		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S3	1	5.600	2100.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	1950.00	1.4	1.0		StrBot: R8-200(B2)
				Support Mc = 2.2		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S4	1	5.600	4050.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2550.00	2.4	1.8		StrBot: R8-200(B2)

Support Mc = 0.3 Support As = 301.43/0.00

Slab Strip: X31 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S6	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	4150.00	6.3	4.8		StrBot: R8-200(B2)
				Support Mc = 6.3		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S5	1	5.600	2989.50	0.0000	0.0000	301.43/251.33 (X)	
	113/150	3.000	1475.00	0.0	0.0		StrBot: R8-200(B2)

Support Mc = 0.0 Support As = 301.43/0.00

Slab Strip: X32 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
3S7	1 121/150	5.600 3.000	1203.60 6300.00	0.0860 1.4	0.0860 1.4	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 2.4			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S8	1 113/150	5.600 3.000	2846.40 2675.00	0.0320 2.6	0.0240 2.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 5.4			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S6	1 113/150	5.600 3.000	4300.00 4150.00	0.0320 6.3	0.0240 4.8	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 5.0			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S17	1 113/150	5.600 3.000	4950.00 2675.00	0.0320 2.6	0.0240 2.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.3			Support As = 301.43/0.00	

Slab Strip: X33 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.0			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S16	1 113/150	5.600 3.000	4950.00 2150.00	0.0000 0.0	0.0000 0.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.0			Support As = 301.43/0.00	

Slab Strip: X34 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2			Support As = 301.43/201.06 (X)	SupTop: R8-

250(T1)

3S9	1	5.600	2846.40	0.0000	0.0240	301.43/251.33 (X)	
	113/150	3.000	2150.00	0.2	1.3		StrBot: R8-200(B2)

Support Mc = 0.2 Support As = 301.43/0.00

Slab Strip: X35 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.0Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

3S7	1	5.600	6300.00	0.0000	0.0000	301.43/251.33 (X)	
	113/150	3.000	1203.60	0.0	0.0		StrBot: R8-200(B2)

Support Mc = 0.0 Support As = 301.43/0.00

Slab Strip: X36 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.1Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

3S10	1	5.600	2846.40	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	1475.00	0.8	0.6		StrBot: R8-200(B2)

Support Mc = 3.2Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

3S12	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	3325.00	4.1	3.1		StrBot: R8-200(B2)

Support Mc = 3.5Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

3S18	1	5.600	2989.50	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2375.00	2.1	1.6		StrBot: R8-200(B2)

Support Mc = 0.2 Support As = 301.43/0.00

Slab Strip: X37 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h	g q	L1 L2	C-sup M-sup	C-span M-span	As Req/Sup	S T E E L B A R S

		(mm)	(kN/m2)	(mm)	(kN.m)	(kN.m)	(mm2)	
				Support Mc = 0.8		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
3S11		1	5.600	4050.00	0.0394	0.0299	322.78/287.23 (X)	
	121/150		3.000	4650.00	7.4	5.6		StrBot: R8-175(B1)
				Support Mc = 6.4		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
3S14		1	5.600	2200.00	0.0530	0.0400	322.78/287.23 (X)	
	121/150		3.000	3300.00	2.9	2.2		StrBot: R8-175(B1)
				Support Mc = 2.9		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
3S13		1	5.600	2100.00	0.0547	0.0411	322.78/287.23 (X)	
	121/150		3.000	3300.00	2.8	2.1		StrBot: R8-175(B1)
				Support Mc = 4.6		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
3S15		1	5.600	4050.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150		3.000	3750.00	5.2	3.9		StrBot: R8-200(B2)
				Support Mc = 0.6		Support As = 301.43/0.00		

Slab Strip: X38 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
3S12		1	5.600	3425.00	0.0442	0.0337 322.78/287.23 (X)	
	121/150		3.000	4300.00	6.0	4.5	StrBot: R8-175(B1)
				Support Mc = 4.6		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
3S14		1	5.600	3300.00	0.0320	0.0240 301.43/251.33 (X)	
	113/150		3.000	2200.00	1.8	1.3	StrBot: R8-200(B2)
				Support Mc = 0.2		Support As = 301.43/0.00	

Slab Strip: X39 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.2		Support As = 301.43/201.06 (X)	SupTop: R8-

250(T1)

3S13	1	5.600	3300.00	0.0000	0.0240	301.43/251.33 (X)
	113/150	3.000	2100.00	0.2	1.2	StrBot: R8-200(B2)

Support Mc = 0.2 Support As = 301.43/0.00

Slab Strip: X40 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

3S3	1	5.600	1950.00	0.0325	0.0250	322.78/287.23 (X)
	121/150	3.000	2000.00	1.4	1.1	StrBot: R8-175(B1)

Support Mc = 5.5Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

3S6	1	5.600	4150.00	0.0332	0.0254	322.78/287.23 (X)
	121/150	3.000	4300.00	6.6	5.0	StrBot: R8-175(B1)

Support Mc = 0.8 Support As = 322.78/0.00

Slab Strip: X41 -- Storey: 3

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

3S2	1	5.600	1950.00	0.0356	0.0271	322.78/287.23 (X)
	121/150	3.000	2100.00	1.6	1.2	StrBot: R8-175(B1)

Support Mc = 5.5Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

3S6	1	5.600	4150.00	0.0332	0.0254	322.78/287.23 (X)
	121/150	3.000	4300.00	6.6	5.0	StrBot: R8-175(B1)

Support Mc = 0.8 Support As = 322.78/0.00

Slab Strip: X41 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h	g q L2	L1 C-sup M-sup	C-span M-span	As Req/Sup	S T E E L B A R S

		(mm)	(kN/m2)		(mm)	(kN.m)	(kN.m)	(mm2)	
					Support Mc = 0.7		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S1		1	5.600		4025.00	0.0314	0.0242	322.78/287.23 (X)	
	121/150		3.000		4050.00	5.8	4.5		StrBot: R8-175(B1)
					Support Mc = 5.1		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S8		1	5.600		2675.00	0.0348	0.0266	322.78/287.23 (X)	
	121/150		3.000		2846.40	2.9	2.2		StrBot: R8-175(B1)
					Support Mc = 2.9		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S9		1	5.600		2150.00	0.0470	0.0355	322.78/287.23 (X)	
	121/150		3.000		2846.40	2.5	1.9		StrBot: R8-175(B1)
					Support Mc = 2.2		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S10		1	5.600		1475.00	0.0619	0.0469	322.78/287.23 (X)	
	121/150		3.000		2846.40	1.5	1.2		StrBot: R8-175(B1)
					Support Mc = 5.4		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
4S11		1	5.600		4650.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150		3.000		4050.00	6.0	4.5		StrBot: R8-200(B2)
					Support Mc = 0.7		Support As = 301.43/0.00		

Slab Strip: X42 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.5		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S4	1	5.600	2550.00	0.0551	0.0414	322.78/287.23 (X)	
	121/150	3.000	4050.00	4.1	3.1		StrBot: R8-175(B1)
			Support Mc = 3.5		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S5	1	5.600	1475.00	0.0630	0.0630	322.78/287.23 (X)	
	121/150	3.000	2989.50	1.6	1.6		StrBot: R8-175(B1)
			Support Mc = 4.2		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
4S17	1	5.600	2675.00	0.0606	0.0456	322.78/287.23 (X)	
	121/150	3.000	4950.00	5.0	3.8		StrBot: R8-175(B1)
			Support Mc = 4.5		Support As = 322.78/251.33 (X)		SupTop: R8-

200(T1)

4S16	1	5.600	2150.00	0.0630	0.0630	322.78/287.23 (X)	
	121/150	3.000	4950.00	3.3	3.3		StrBot: R8-175(B1)

Support Mc = 3.3Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

4S18	1	5.600	2375.00	0.0443	0.0338	322.78/287.23 (X)	
	121/150	3.000	2989.50	2.9	2.2		StrBot: R8-175(B1)

Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

4S15	1	5.600	3750.00	0.0358	0.0272	322.78/287.23 (X)	
	121/150	3.000	4050.00	5.8	4.4		StrBot: R8-175(B1)

Support Mc = 0.7 Support As = 322.78/0.00

Slab Strip: X43 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.0Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

4S7	1	5.600	6300.00	0.0000	0.0000	301.43/251.33 (X)	
	113/150	3.000	1203.60	0.0	0.0		StrBot: R8-200(B2)

Support Mc = 0.0 Support As = 301.43/0.00

Slab Strip: X44 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

4S2	1	5.600	1950.00	0.0356	0.0271	322.78/287.23 (X)	
	121/150	3.000	2100.00	1.6	1.2		StrBot: R8-175(B1)

Support Mc = 5.5Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

4S6	1	5.600	4150.00	0.0332	0.0254	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.6	5.0		StrBot: R8-175(B1)

Support Mc = 0.8 Support As = 322.78/0.00

Slab Strip: X45 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.2		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S3	1	5.600	1950.00	0.0325	0.0250	322.78/287.23 (X)	
	121/150	3.000	2000.00	1.4	1.1		StrBot: R8-175(B1)
				Support Mc = 5.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S6	1	5.600	4150.00	0.0332	0.0254	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.6	5.0		StrBot: R8-175(B1)
				Support Mc = 0.8		Support As = 322.78/0.00	

Slab Strip: X46 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S12	1	5.600	3425.00	0.0442	0.0337	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.0	4.5		StrBot: R8-175(B1)
				Support Mc = 4.6		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S14	1	5.600	3300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2200.00	1.8	1.3		StrBot: R8-200(B2)
				Support Mc = 0.2		Support As = 301.43/0.00	

Slab Strip: X47 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S12	1	5.600	3425.00	0.0442	0.0337	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.0	4.5		StrBot: R8-175(B1)
				Support Mc = 4.5		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)

```
 4S13            1      5.600    3300.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    2100.00        1.6        1.2                   StrBot: R8-200(B2)

                                  Support Mc = 0.2 Support As = 301.43/0.00


Slab Strip: X48 -- Storey: 4
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Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span           As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 4S1             1      5.600    4050.00     0.0320     0.0240 322.78/287.23 (X)
          121/150       3.000    4025.00        6.0        4.5                   StrBot: R8-175(B1)

                                  Support Mc = 4.9Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 4S2             1      5.600    2200.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    1950.00        1.4        1.0                   StrBot: R8-200(B2)

                                  Support Mc = 1.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 4S3             1      5.600    2100.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    1950.00        1.4        1.0                   StrBot: R8-200(B2)

                                  Support Mc = 2.2Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 4S4             1      5.600    4050.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    2550.00        2.4        1.8                   StrBot: R8-200(B2)

                                  Support Mc = 0.3 Support As = 301.43/0.00


Slab Strip: X49 -- Storey: 4
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span           As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 4S6             1      5.600    4300.00     0.0320     0.0240 301.43/251.33 (X)
          113/150       3.000    4150.00        6.3        4.8                   StrBot: R8-200(B2)

                                  Support Mc = 6.3Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 4S5             1      5.600    2989.50     0.0000     0.0000 301.43/251.33 (X)
          113/150       3.000    1475.00        0.0        0.0                   StrBot: R8-200(B2)
```

Support Mc = 0.0 Support As = 301.43/0.00

Slab Strip: X50 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S7	1 121/150	5.600 3.000	1203.60 6300.00	0.0860 1.4	0.0860 1.4	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 2.4			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S8	1 113/150	5.600 3.000	2846.40 2675.00	0.0320 2.6	0.0240 2.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 5.4			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S6	1 113/150	5.600 3.000	4300.00 4150.00	0.0320 6.3	0.0240 4.8	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 5.0			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S17	1 113/150	5.600 3.000	4950.00 2675.00	0.0320 2.6	0.0240 2.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.3			Support As = 301.43/0.00	

Slab Strip: X51 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S9	1 113/150	5.600 3.000	2846.95 2150.40	0.0000 0.2	0.0240 1.3	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2			Support As = 301.43/0.00	

Slab Strip: X52 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.0			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S16	1 113/150	5.600 3.000	4950.00 2150.00	0.0000 0.0	0.0000 0.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.0			Support As = 301.43/0.00	

Slab Strip: X53 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.1			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S10	1 113/150	5.600 3.000	2846.40 1475.00	0.0320 0.8	0.0240 0.6	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 3.2			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S12	1 113/150	5.600 3.000	4300.00 3325.00	0.0320 4.1	0.0240 3.1	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 3.5			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
4S18	1 113/150	5.600 3.000	2989.50 2375.00	0.0320 2.1	0.0240 1.6	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2			Support As = 301.43/0.00	

Slab Strip: X54 -- Storey: 4

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.8			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S11	1 121/150	5.600 3.000	4050.00 4650.00	0.0394 7.4	0.0299 5.6	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 6.4			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
4S14	1 121/150	5.600 3.000	2200.00 3300.00	0.0530 2.9	0.0400 2.2	322.78/287.23 (X)	StrBot: R8-175(B1)

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                                  Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 4S13            1      5.600    2100.00     0.0547     0.0411 322.78/287.23 (X)
           121/150      3.000    3300.00        2.8        2.1                   StrBot: R8-175(B1)

                                  Support Mc = 4.6Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 4S15            1      5.600    4050.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    3750.00        5.2        3.9                   StrBot: R8-200(B2)

                                  Support Mc = 0.6 Support As = 301.43/0.00


Slab Strip: X54 -- Storey: 5
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S1             1      5.600    4025.00     0.0314     0.0242 322.78/287.23 (X)
           121/150      3.000    4050.00        5.8        4.5                   StrBot: R8-175(B1)

                                  Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S8             1      5.600    2675.00     0.0348     0.0266 322.78/287.23 (X)
           121/150      3.000    2846.40        2.9        2.2                   StrBot: R8-175(B1)

                                  Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S9             1      5.600    2150.00     0.0470     0.0355 322.78/287.23 (X)
           121/150      3.000    2846.40        2.5        1.9                   StrBot: R8-175(B1)

                                  Support Mc = 2.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S10            1      5.600    1475.00     0.0619     0.0469 322.78/287.23 (X)
           121/150      3.000    2846.40        1.5        1.2                   StrBot: R8-175(B1)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S11            1      5.600    4650.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    4050.00        6.0        4.5                   StrBot: R8-200(B2)

                                  Support Mc = 0.7 Support As = 301.43/0.00


Slab Strip: X55 -- Storey: 5
----------------------------
Materials: C25/30 / Grade 250 (Plain)
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Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
5S2	1 121/150	5.600 3.000	1950.00 2100.00	0.0356 1.6	0.0271 1.2	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 5.5		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
5S6	1 121/150	5.600 3.000	4150.00 4300.00	0.0332 6.6	0.0254 5.0	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 0.8		Support As = 322.78/0.00		

Slab Strip: X56 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
5S12	1 121/150	5.600 3.000	3425.00 4300.00	0.0442 6.0	0.0337 4.5	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 4.6		Support As = 301.43/201.06 (X)		SupTop: R8-250(T1)
5S14	1 113/150	5.600 3.000	3300.00 2200.00	0.0320 1.8	0.0240 1.3	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2		Support As = 301.43/0.00		

Slab Strip: X57 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.2		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
5S3	1 121/150	5.600 3.000	1950.00 2000.00	0.0325 1.4	0.0250 1.1	322.78/287.23 (X)	StrBot: R8-175(B1)
			Support Mc = 5.5		Support As = 322.78/251.33 (X)		SupTop: R8-200(T1)
5S6	1 121/150	5.600 3.000	4150.00 4300.00	0.0332 6.6	0.0254 5.0	322.78/287.23 (X)	StrBot: R8-175(B1)

Support Mc = 0.8 Support As = 322.78/0.00

Slab Strip: X58 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.7		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
5S12	1	5.600	3425.00	0.0442	0.0337	322.78/287.23 (X)	
	121/150	3.000	4300.00	6.0	4.5		StrBot: R8-175(B1)
				Support Mc = 4.5		Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
5S13	1	5.600	3300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2100.00	1.6	1.2		StrBot: R8-200(B2)

Support Mc = 0.2 Support As = 301.43/0.00

Slab Strip: X59 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
5S4	1	5.600	2550.00	0.0551	0.0414	322.78/287.23 (X)	
	121/150	3.000	4050.00	4.1	3.1		StrBot: R8-175(B1)
				Support Mc = 3.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
5S5	1	5.600	1475.00	0.0630	0.0630	322.78/287.23 (X)	
	121/150	3.000	2989.50	1.6	1.6		StrBot: R8-175(B1)
				Support Mc = 4.2		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
5S17	1	5.600	2675.00	0.0606	0.0456	322.78/287.23 (X)	
	121/150	3.000	4950.00	5.0	3.8		StrBot: R8-175(B1)
				Support Mc = 4.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
5S16	1	5.600	2150.00	0.0630	0.0630	322.78/287.23 (X)	
	121/150	3.000	4950.00	3.3	3.3		StrBot: R8-175(B1)
				Support Mc = 3.3		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
5S18	1	5.600	2375.00	0.0443	0.0338	322.78/287.23 (X)	
	121/150	3.000	2989.50	2.9	2.2		StrBot: R8-175(B1)
			Support Mc = 5.1	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
5S15	1	5.600	3750.00	0.0358	0.0272	322.78/287.23 (X)	
	121/150	3.000	4050.00	5.8	4.4		StrBot: R8-175(B1)
			Support Mc = 0.7	Support As = 322.78/0.00			

Slab Strip: X60 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7	Support As = 322.78/251.33 (X)			SupTop: R8-200(T1)
5S1	1	5.600	4050.00	0.0320	0.0240	322.78/287.23 (X)	
	121/150	3.000	4025.00	6.0	4.5		StrBot: R8-175(B1)
			Support Mc = 4.9	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
5S2	1	5.600	2200.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	1950.00	1.4	1.0		StrBot: R8-200(B2)
			Support Mc = 1.4	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
5S3	1	5.600	2100.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	1950.00	1.4	1.0		StrBot: R8-200(B2)
			Support Mc = 2.2	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
5S4	1	5.600	4050.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2550.00	2.4	1.8		StrBot: R8-200(B2)
			Support Mc = 0.3	Support As = 301.43/0.00			

Slab Strip: X61 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.7	Support As = 301.43/201.06 (X)			SupTop: R8-250(T1)
5S6	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	4150.00	6.3	4.8		StrBot: R8-200(B2)

```
                                  Support Mc = 6.3Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S5             1      5.600    2989.50     0.0000     0.0000 301.43/251.33 (X)
           113/150      3.000    1475.00        0.0        0.0                    StrBot: R8-200(B2)

                                  Support Mc = 0.0 Support As = 301.43/0.00


Slab Strip: X62 -- Storey: 5
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S7             1      5.600    1203.60     0.0860     0.0860 322.78/287.23 (X)
           121/150      3.000    6300.00        1.4        1.4                    StrBot: R8-175(B1)

                                  Support Mc = 2.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S8             1      5.600    2846.40     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    2675.00        2.6        2.0                    StrBot: R8-200(B2)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S6             1      5.600    4300.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    4150.00        6.3        4.8                    StrBot: R8-200(B2)

                                  Support Mc = 5.0Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S17            1      5.600    4950.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    2675.00        2.6        2.0                    StrBot: R8-200(B2)

                                  Support Mc = 0.3 Support As = 301.43/0.00


Slab Strip: X63 -- Storey: 5
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.2Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S9             1      5.600    2846.40     0.0000     0.0240 301.43/251.33 (X)
           113/150      3.000    2150.00        0.2        1.3                    StrBot: R8-200(B2)

                                  Support Mc = 0.2 Support As = 301.43/0.00
```

Slab Strip: X64 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.0			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
5S16	1 113/150	5.600 3.000	4950.00 2150.00	0.0000 0.0	0.0000 0.0	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.0			Support As = 301.43/0.00	

Slab Strip: X65 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.1			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
5S10	1 113/150	5.600 3.000	2846.40 1475.00	0.0320 0.8	0.0240 0.6	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 3.2			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
5S12	1 113/150	5.600 3.000	4300.00 3325.00	0.0320 4.1	0.0240 3.1	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 3.5			Support As = 301.43/201.06 (X)	SupTop: R8-250(T1)
5S18	1 113/150	5.600 3.000	2989.50 2375.00	0.0320 2.1	0.0240 1.6	301.43/251.33 (X)	StrBot: R8-200(B2)
			Support Mc = 0.2			Support As = 301.43/0.00	

Slab Strip: X66 -- Storey: 5

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
			Support Mc = 0.8			Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
5S11	1 121/150	5.600 3.000	4050.00 4650.00	0.0394 7.4	0.0299 5.6	322.78/287.23 (X)	StrBot: R8-175(B1)

```
                                  Support Mc = 6.4Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S14            1      5.600    2200.00     0.0530     0.0400 322.78/287.23 (X)
           121/150      3.000    3300.00        2.9        2.2                   StrBot: R8-175(B1)

                                  Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 5S13            1      5.600    2100.00     0.0547     0.0411 322.78/287.23 (X)
           121/150      3.000    3300.00        2.8        2.1                   StrBot: R8-175(B1)

                                  Support Mc = 4.6Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 5S15            1      5.600    4050.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    3750.00        5.2        3.9                   StrBot: R8-200(B2)

                                  Support Mc = 0.6 Support As = 301.43/0.00


Slab Strip: X66 -- Storey: 6
----------------------------
Materials: C25/30 / Grade 250 (Plain)


              Type          g         L1      C-sup     C-span            As
 Slab          d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
              (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S1             1      5.600    4025.00     0.0314     0.0242 322.78/287.23 (X)
           121/150      3.000    4050.00        5.8        4.5                   StrBot: R8-175(B1)

                                  Support Mc = 5.1Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S8             1      5.600    2675.00     0.0348     0.0266 322.78/287.23 (X)
           121/150      3.000    2846.40        2.9        2.2                   StrBot: R8-175(B1)

                                  Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S9             1      5.600    2150.00     0.0470     0.0355 322.78/287.23 (X)
           121/150      3.000    2846.40        2.5        1.9                   StrBot: R8-175(B1)

                                  Support Mc = 2.2Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S10            1      5.600    1475.00     0.0619     0.0469 322.78/287.23 (X)
           121/150      3.000    2846.40        1.5        1.2                   StrBot: R8-175(B1)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S11            1      5.600    4650.00     0.0320     0.0240 301.43/251.33 (X)
           113/150      3.000    4050.00        6.0        4.5                   StrBot: R8-200(B2)

                                  Support Mc = 0.7 Support As = 301.43/0.00
```

Slab Strip: X67 -- Storey: 6

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S4	1	5.600	2550.00	0.0551	0.0414	322.78/287.23 (X)	
	121/150	3.000	4050.00	4.1	3.1		StrBot: R8-175(B1)
				Support Mc = 3.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S5	1	5.600	1475.00	0.0630	0.0630	322.78/287.23 (X)	
	121/150	3.000	2989.50	1.6	1.6		StrBot: R8-175(B1)
				Support Mc = 4.2		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S17	1	5.600	2675.00	0.0606	0.0456	322.78/287.23 (X)	
	121/150	3.000	4950.00	5.0	3.8		StrBot: R8-175(B1)
				Support Mc = 4.5		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S16	1	5.600	2150.00	0.0630	0.0630	322.78/287.23 (X)	
	121/150	3.000	4950.00	3.3	3.3		StrBot: R8-175(B1)
				Support Mc = 3.3		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S18	1	5.600	2375.00	0.0443	0.0338	322.78/287.23 (X)	
	121/150	3.000	2989.50	2.9	2.2		StrBot: R8-175(B1)
				Support Mc = 5.1		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S15	1	5.600	3750.00	0.0358	0.0272	322.78/287.23 (X)	
	121/150	3.000	4050.00	5.8	4.4		StrBot: R8-175(B1)
				Support Mc = 0.7		Support As = 322.78/0.00	

Slab Strip: X68 -- Storey: 6

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S
				Support Mc = 0.2		Support As = 322.78/251.33 (X)	SupTop: R8-200(T1)
6S2	1	5.600	1950.00	0.0356	0.0271	322.78/287.23 (X)	

```
          121/150      3.000    2100.00        1.6        1.2                    StrBot: R8-175(B1)

                                  Support Mc = 5.5Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S6            1      5.600    4150.00     0.0332     0.0254 322.78/287.23 (X)
          121/150      3.000    4300.00        6.6        5.0                    StrBot: R8-175(B1)

                                  Support Mc = 5.9Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S20           1      5.600    2150.00     0.0630     0.0480 322.78/287.23 (X)
          121/150      3.000    4300.00        3.3        2.6                    StrBot: R8-175(B1)

                                  Support Mc = 5.3Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S12           1      5.600    3425.00     0.0442     0.0337 322.78/287.23 (X)
          121/150      3.000    4300.00        6.0        4.5                    StrBot: R8-175(B1)

                                  Support Mc = 4.6Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S14           1      5.600    3300.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    2200.00        1.8        1.3                    StrBot: R8-200(B2)

                                  Support Mc = 0.2 Support As = 301.43/0.00


Slab Strip: X69 -- Storey: 6
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span            As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.0Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S7            1      5.600    6300.00     0.0000     0.0000 301.43/251.33 (X)
          113/150      3.000    1203.60        0.0        0.0                    StrBot: R8-200(B2)

                                  Support Mc = 0.0 Support As = 301.43/0.00


Slab Strip: X70 -- Storey: 6
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span            As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.7Support As = 322.78/251.33 (X) SupTop: R8-
200(T1)

 6S1            1      5.600    4050.00     0.0320     0.0240 322.78/287.23 (X)
          121/150      3.000    4025.00        6.0        4.5                    StrBot: R8-175(B1)

                                  Support Mc = 4.9Support As = 301.43/201.06 (X) SupTop: R8-
```

250(T1)

6S2	1	5.600	2200.00	0.0320	0.0240	301.43/251.33 (X)
	113/150	3.000	1950.00	1.4	1.0	StrBot: R8-200(B2)

Support Mc = 1.4Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S3	1	5.600	2100.00	0.0320	0.0240	301.43/251.33 (X)
	113/150	3.000	1950.00	1.4	1.0	StrBot: R8-200(B2)

Support Mc = 2.2Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S4	1	5.600	4050.00	0.0320	0.0240	301.43/251.33 (X)
	113/150	3.000	2550.00	2.4	1.8	StrBot: R8-200(B2)

Support Mc = 0.3 Support As = 301.43/0.00

Slab Strip: X71 -- Storey: 6

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.7Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S6	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)
	113/150	3.000	4150.00	6.3	4.8	StrBot: R8-200(B2)

Support Mc = 6.3Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S5	1	5.600	2989.50	0.0000	0.0000	301.43/251.33 (X)
	113/150	3.000	1475.00	0.0	0.0	StrBot: R8-200(B2)

Support Mc = 0.0 Support As = 301.43/0.00

Slab Strip: X72 -- Storey: 6

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.2Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

6S7	1	5.600	1203.60	0.0860	0.0860	322.78/287.23 (X)
	121/150	3.000	6300.00	1.4	1.4	StrBot: R8-175(B1)

Support Mc = 2.4Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S8	1	5.600	2846.40	0.0320	0.0240	301.43/251.33 (X)

```
          113/150      3.000    2675.00        2.6        2.0                    StrBot: R8-200(B2)

                                  Support Mc = 5.4Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S6            1      5.600    4300.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    4150.00        6.3        4.8                    StrBot: R8-200(B2)

                                  Support Mc = 5.0Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S17           1      5.600    4950.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    2675.00        2.6        2.0                    StrBot: R8-200(B2)

                                  Support Mc = 0.3 Support As = 301.43/0.00


Slab Strip: X73 -- Storey: 6
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span            As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.2Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S9            1      5.600    2846.40     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    2150.00        1.7        1.3                    StrBot: R8-200(B2)

                                  Support Mc = 1.7Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S20           1      5.600    4300.00     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    2150.00        1.7        1.3                    StrBot: R8-200(B2)

                                  Support Mc = 1.7Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S16           1      5.600    4950.00     0.0000     0.0000 301.43/251.33 (X)
          113/150      3.000    2150.00        0.0        0.0                    StrBot: R8-200(B2)

                                  Support Mc = 0.0 Support As = 301.43/0.00


Slab Strip: X74 -- Storey: 6
----------------------------
Materials: C25/30 / Grade 250 (Plain)


             Type          g         L1      C-sup     C-span            As
 Slab         d/h          q         L2      M-sup     M-span       Req/Sup   S T E E L   B A R S
             (mm)     (kN/m2)       (mm)     (kN.m)     (kN.m)         (mm2)

                                  Support Mc = 0.1Support As = 301.43/201.06 (X) SupTop: R8-
250(T1)

 6S10           1      5.600    2846.40     0.0320     0.0240 301.43/251.33 (X)
          113/150      3.000    1475.00        0.8        0.6                    StrBot: R8-200(B2)

                                  Support Mc = 3.2Support As = 301.43/201.06 (X) SupTop: R8-
```

250(T1)

6S12	1	5.600	4300.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	3325.00	4.1	3.1		StrBot: R8-200(B2)

Support Mc = 3.5Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S18	1	5.600	2989.50	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	2375.00	2.1	1.6		StrBot: R8-200(B2)

Support Mc = 0.2 Support As = 301.43/0.00

Slab Strip: X75 -- Storey: 6

Materials: C25/30 / Grade 250 (Plain)

Slab	Type d/h (mm)	g q (kN/m2)	L1 L2 (mm)	C-sup M-sup (kN.m)	C-span M-span (kN.m)	As Req/Sup (mm2)	S T E E L B A R S

Support Mc = 0.8Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

6S11	1	5.600	4050.00	0.0394	0.0299	322.78/287.23 (X)	
	121/150	3.000	4650.00	7.4	5.6		StrBot: R8-175(B1)

Support Mc = 6.4Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

6S14	1	5.600	2200.00	0.0530	0.0400	322.78/287.23 (X)	
	121/150	3.000	3300.00	2.9	2.2		StrBot: R8-175(B1)

Support Mc = 2.9Support As = 322.78/251.33 (X) SupTop: R8-200(T1)

6S13	1	5.600	2100.00	0.0547	0.0411	322.78/287.23 (X)	
	121/150	3.000	3300.00	2.8	2.1		StrBot: R8-175(B1)

Support Mc = 4.6Support As = 301.43/201.06 (X) SupTop: R8-250(T1)

6S15	1	5.600	4050.00	0.0320	0.0240	301.43/251.33 (X)	
	113/150	3.000	3750.00	5.2	3.9		StrBot: R8-200(B2)

Support Mc = 0.6 Support As = 301.43/0.00

Slab Deflection Check

1S1 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.8 / 4.4 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.179 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S2 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.010 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S3 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.5 / 0.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.009 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S4 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2350.0 / 150.0 mm = 15.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.0 / 3.1 kN.m	1.5 / 2.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.046 mm ≤ 6.528 mm (2350/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

1S5 - Storey: 1

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE

Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.006 mm ≤ 3.542 mm (1275/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

1S6 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3950.0 / 150.0 mm = 26.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	3.2 / 4.9 kN.m	0.0 / 0.0 kN.m	2.5 / 3.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.213 mm ≤ 10.972 mm (3950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

1S7 - Storey: 1

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 978.6 / 150.0 mm = 6.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.7 / 1.1 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 2.718 mm (979/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S8 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.031 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S9 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.2 / 1.9 kN.m	1.2 / 1.9 kN.m	0.9 / 1.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.016 mm ≤ 5.417 mm (1950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S10 - Storey: 1

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 3.542 mm (1275/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S11 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	3.6 / 5.6 kN.m	2.7 / 4.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.224 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S12 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3225.0 / 150.0 mm = 21.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.9 / 4.5 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.128 mm ≤ 8.958 mm (3225/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

1S13 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1900.0 / 150.0 mm = 12.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.0 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.017 mm ≤ 5.278 mm (1900/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S14 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2000.0 / 150.0 mm = 13.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.7 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.020 mm ≤ 5.556 mm (2000/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S15 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3550.0 / 150.0 mm = 23.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	2.8 / 4.3 kN.m	0.0 / 0.0 kN.m	2.1 / 3.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.150 mm ≤ 9.861 mm (3550/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

1S16 - Storey: 1

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.032 mm ≤ 5.417 mm (1950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S17 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.4 / 3.7 kN.m	2.4 / 3.7 kN.m	1.8 / 2.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.053 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

1S18 - Storey: 1

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2175.0 / 150.0 mm = 14.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.024 mm ≤ 6.042 mm (2175/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S1 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.8 / 4.4 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.179 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S2 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.010 mm ≤ 4.861 mm (1750/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

2S3 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Simple Support
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 25 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.0 / 0.0 kN.m	0.5 / 0.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.010 mm ≤ 4.861 mm (1750/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

2S4 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2350.0 / 150.0 mm = 15.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.0 / 3.1 kN.m	1.5 / 2.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.046 mm ≤ 6.528 mm (2350/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

2S5 - Storey: 2

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.006 mm ≤ 3.542 mm (1275/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S6 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3950.0 / 150.0 mm = 26.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	3.2 / 4.9 kN.m	0.0 / 0.0 kN.m	2.5 / 3.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.213 mm ≤ 10.972 mm (3950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S7 - Storey: 2

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 978.6 / 150.0 mm = 6.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.7 / 1.1 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 2.718 mm (979/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S8 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.031 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S9 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.2 / 1.9 kN.m	1.2 / 1.9 kN.m	0.9 / 1.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.016 mm ≤ 5.417 mm (1950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S10 - Storey: 2

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 3.542 mm (1275/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

2S11 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	3.6 / 5.6 kN.m	2.7 / 4.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.224 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S12 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3225.0 / 150.0 mm = 21.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.9 / 4.5 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.128 mm ≤ 8.958 mm (3225/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S13 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1900.0 / 150.0 mm = 12.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.0 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.017 mm ≤ 5.278 mm (1900/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

2S14 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2000.0 / 150.0 mm = 13.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.7 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.020 mm ≤ 5.556 mm (2000/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S15 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3550.0 / 150.0 mm = 23.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.8 / 4.3 kN.m	0.0 / 0.0 kN.m	2.1 / 3.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.150 mm ≤ 9.861 mm (3550/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S16 - Storey: 2

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.032 mm ≤ 5.417 mm (1950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S17 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.4 / 3.7 kN.m	2.4 / 3.7 kN.m	1.8 / 2.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.053 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

2S18 - Storey: 2

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2175.0 / 150.0 mm = 14.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.024 mm ≤ 6.042 mm (2175/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S1 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.8 / 4.4 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.179 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S2 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.010 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S3 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.5 / 0.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.009 mm ≤ 4.861 mm (1750/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S4 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2350.0 / 150.0 mm = 15.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.0 / 3.1 kN.m	1.5 / 2.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.046 mm ≤ 6.528 mm (2350/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S5 - Storey: 3

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.006 mm ≤ 3.542 mm (1275/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S6 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3950.0 / 150.0 mm = 26.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	3.2 / 4.9 kN.m	0.0 / 0.0 kN.m	2.5 / 3.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.213 mm ≤ 10.972 mm (3950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S7 - Storey: 3

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 978.6 / 150.0 mm = 6.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.7 / 1.1 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 2.718 mm (979/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S8 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.031 mm ≤ 6.875 mm (2475/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S9 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.2 / 1.9 kN.m	1.2 / 1.9 kN.m	0.9 / 1.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.016 mm ≤ 5.417 mm (1950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S10 - Storey: 3

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 3.542 mm (1275/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S11 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	3.6 / 5.6 kN.m	2.7 / 4.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.224 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S12 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3225.0 / 150.0 mm = 21.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.9 / 4.5 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.128 mm ≤ 8.958 mm (3225/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S13 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1900.0 / 150.0 mm = 12.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.0 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.017 mm ≤ 5.278 mm (1900/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S14 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2000.0 / 150.0 mm = 13.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.7 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.020 mm ≤ 5.556 mm (2000/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S15 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3550.0 / 150.0 mm = 23.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.8 / 4.3 kN.m	0.0 / 0.0 kN.m	2.1 / 3.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.150 mm ≤ 9.861 mm (3550/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

3S16 - Storey: 3

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.032 mm ≤ 5.417 mm (1950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S17 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.4 / 3.7 kN.m	2.4 / 3.7 kN.m	1.8 / 2.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.053 mm ≤ 6.875 mm (2475/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

3S18 - Storey: 3

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2175.0 / 150.0 mm = 14.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.024 mm ≤ 6.042 mm (2175/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

4S1 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.8 / 4.4 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.179 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S2 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.010 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S3 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.5 / 0.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.009 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S4 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2350.0 / 150.0 mm = 15.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.0 / 3.1 kN.m	1.5 / 2.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.046 mm ≤ 6.528 mm (2350/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S5 - Storey: 4

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.006 mm ≤ 3.542 mm (1275/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S6 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3950.0 / 150.0 mm = 26.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	3.2 / 4.9 kN.m	0.0 / 0.0 kN.m	2.5 / 3.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.213 mm ≤ 10.972 mm (3950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S7 - Storey: 4

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 978.6 / 150.0 mm = 6.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.7 / 1.1 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 2.718 mm (979/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S8 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.031 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S9 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.2 / 1.9 kN.m	1.2 / 1.9 kN.m	0.9 / 1.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.016 mm ≤ 5.417 mm (1950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

4S10 - Storey: 4

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 3.542 mm (1275/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S11 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	3.6 / 5.6 kN.m	2.7 / 4.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.224 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S12 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3225.0 / 150.0 mm = 21.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.9 / 4.5 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.128 mm ≤ 8.958 mm (3225/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S13 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1900.0 / 150.0 mm = 12.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.0 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.017 mm ≤ 5.278 mm (1900/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

4S14 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2000.0 / 150.0 mm = 13.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.7 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.020 mm ≤ 5.556 mm (2000/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

4S15 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3550.0 / 150.0 mm = 23.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.8 / 4.3 kN.m	0.0 / 0.0 kN.m	2.1 / 3.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.150 mm ≤ 9.861 mm (3550/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

4S16 - Storey: 4

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.032 mm ≤ 5.417 mm (1950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

4S17 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	2.4 / 3.7 kN.m	2.4 / 3.7 kN.m	1.8 / 2.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.053 mm ≤ 6.875 mm (2475/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

4S18 - Storey: 4

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2175.0 / 150.0 mm = 14.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.024 mm ≤ 6.042 mm (2175/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S1 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.8 / 4.4 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.179 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S2 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.010 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S3 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.5 / 0.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.009 mm ≤ 4.861 mm (1750/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S4 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2350.0 / 150.0 mm = 15.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.0 / 3.1 kN.m	1.5 / 2.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.046 mm ≤ 6.528 mm (2350/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

5S5 - Storey: 5

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.006 mm ≤ 3.542 mm (1275/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S6 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3950.0 / 150.0 mm = 26.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	3.2 / 4.9 kN.m	0.0 / 0.0 kN.m	2.5 / 3.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.213 mm ≤ 10.972 mm (3950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S7 - Storey: 5

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 978.6 / 150.0 mm = 6.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.7 / 1.1 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 2.718 mm (979/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S8 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.031 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S9 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.2 / 1.9 kN.m	1.2 / 1.9 kN.m	0.9 / 1.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.016 mm ≤ 5.417 mm (1950/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S10 - Storey: 5

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.004 mm ≤ 3.542 mm (1275/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S11 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	3.6 / 5.6 kN.m	2.7 / 4.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.224 mm ≤ 10.625 mm (3825/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S12 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3225.0 / 150.0 mm = 21.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.9 / 4.5 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.128 mm ≤ 8.958 mm (3225/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

5S13 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1900.0 / 150.0 mm = 12.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.0 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.017 mm ≤ 5.278 mm (1900/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

5S14 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2000.0 / 150.0 mm = 13.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.7 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.020 mm ≤ 5.556 mm (2000/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

5S15 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3550.0 / 150.0 mm = 23.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	2.8 / 4.3 kN.m	0.0 / 0.0 kN.m	2.1 / 3.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.150 mm ≤ 9.861 mm (3550/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

5S16 - Storey: 5

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.032 mm ≤ 5.417 mm (1950/360) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S17 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.4 / 3.7 kN.m	2.4 / 3.7 kN.m	1.8 / 2.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.053 mm ≤ 6.875 mm (2475/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

5S18 - Storey: 5

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2175.0 / 150.0 mm = 14.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Member Not Supporting Partition Wall
Immediate Def. (Q) = 0.024 mm ≤ 6.042 mm (2175/360) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S14 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2000.0 / 150.0 mm = 13.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.7 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.020 mm ≤ 11.111 mm (2000/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S13 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1900.0 / 150.0 mm = 12.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.0 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.017 mm ≤ 10.556 mm (1900/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S15 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3550.0 / 150.0 mm = 23.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.8 / 4.3 kN.m	0.0 / 0.0 kN.m	2.1 / 3.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.150 mm ≤ 19.722 mm (3550/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S18 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2175.0 / 150.0 mm = 14.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.2 kN.m	1.4 / 2.2 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.024 mm ≤ 12.083 mm (2175/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S12 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3225.0 / 150.0 mm = 21.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	2.9 / 4.5 kN.m	2.9 / 4.5 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.109 mm ≤ 17.917 mm (3225/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S11 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	3.6 / 5.6 kN.m	2.7 / 4.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.224 mm ≤ 21.250 mm (3825/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S10 - Storey: 6

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	335.10 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.004 mm ≤ 7.083 mm (1275/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S9 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.2 / 1.9 kN.m	1.2 / 1.9 kN.m	0.9 / 1.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.016 mm ≤ 10.833 mm (1950/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S16 - Storey: 6

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.032 mm ≤ 10.833 mm (1950/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S17 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	2.4 / 3.7 kN.m	2.4 / 3.7 kN.m	1.8 / 2.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.053 mm ≤ 13.750 mm (2475/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S5 - Storey: 6

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1275.0 / 150.0 mm = 8.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m	0.8 / 1.2 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.006 mm ≤ 7.083 mm (1275/180) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

6S8 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2475.0 / 150.0 mm = 16.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.4 / 2.1 kN.m	1.4 / 2.1 kN.m	1.1 / 1.6 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.031 mm ≤ 13.750 mm (2475/180) **... Adequate** √

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** √

6S7 - Storey: 6

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 978.6 / 150.0 mm = 6.5 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span

$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	335.10 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.7 / 1.1 kN.m	0.7 / 1.1 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.004 mm ≤ 5.437 mm (979/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm²	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S6 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3950.0 / 150.0 mm = 26.3 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m⁴	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm²	F_{ctd} = 1.17 N/mm²	E_c = 31000.0 N/mm²
Steel:	F_{yd} = 217.39 N/mm²	E_s = 200000.0 N/mm²	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm²	402.12 / 335.10 mm²	0.00 / 335.10 mm²
M(g) / M(g+q)	3.2 / 4.9 kN.m	3.2 / 4.9 kN.m	2.5 / 3.8 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴
Effective Inertia (G)	0.000281 m⁴	0.000281 m⁴	0.000281 m⁴

Effective Inertia (G)	0.000281 m⁴
Effective Inertia (G+Q)	0.000281 m⁴
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.181 mm ≤ 21.944 mm (3950/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm²	Steel Bar Stress	152.17 N/mm²
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0

Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S4 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 2350.0 / 150.0 mm = 15.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.0 / 3.1 kN.m	1.5 / 2.3 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.046 mm ≤ 13.056 mm (2350/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S2 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 1750.0 / 150.0 mm = 11.7 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	0.8 / 1.2 kN.m	0.6 / 0.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.010 mm ≤ 9.722 mm (1750/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S1 - Storey: 6

Flexural Member Type : Two-way Slab
Span Type : Interior Span
Clear Span / Height : 3825.0 / 150.0 mm = 25.5 ≤ 35 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	0.0 / 0.0 kN.m	2.8 / 4.4 kN.m	2.2 / 3.4 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.001 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.001 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.179 mm ≤ 21.250 mm (3825/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

6S20 - Storey: 6

Flexural Member Type : One-way Slab
Span Type : Interior Span
Clear Span / Height : 1950.0 / 150.0 mm = 13.0 ≤ 30 (TS-500 Table 13.1)

Section Dimensions:	1000.0 / 150.0 mm	d = 121.0 mm	d' = 29.0 mm
Flange Dimensions:	b = 1000.0 mm	h_f = 0.0 mm	
Uncracked Section:	I_c = 0.000281 m^4	y(b) = 75.0 mm	y(t) = 75.0 mm
Concrete:	F_{cd} = 16.67 N/mm^2	F_{ctd} = 1.17 N/mm^2	E_c = 31000.0 N/mm^2
Steel:	F_{yd} = 217.39 N/mm^2	E_s = 200000.0 N/mm^2	E_s/E_c = 6.5

	Left Support	Right Support	Span
$A_s(t)/A_s(b)$	402.12 / 335.10 mm^2	402.12 / 335.10 mm^2	0.00 / 335.10 mm^2
M(g) / M(g+q)	1.6 / 2.5 kN.m	1.6 / 2.5 kN.m	1.2 / 1.9 kN.m
M_{cr}	10.9 kN.m	10.9 kN.m	10.9 kN.m
Cracking Status (G)	NO	NO	NO
Cracking Status (G+Q)	NO	NO	NO
Cracked Inertia (G)	-	-	-
Cracked Inertia (G+Q)	-	-	-
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4
Effective Inertia (G)	0.000281 m^4	0.000281 m^4	0.000281 m^4

Effective Inertia (G)	0.000281 m^4
Effective Inertia (G+Q)	0.000281 m^4
Immediate Def. (G)	0.000 m
Immediate Def. (G+Q)	0.000 m
Immediate Def. (Q)	0.000 m
Loading Time	FIVE YEARS OR MORE
Total Time Dep. Deflection	0.000 m

Flexural Member and Position: Roof Member not Supporting Partitions
Immediate Def. (Q) = 0.022 mm ≤ 10.833 mm (1950/180) **... Adequate** ✓

Crack Width Check (Span)::

Required/Supplied A_s	335.10 / 322.78 mm^2	Steel Bar Stress	152.17 N/mm^2
No.of Tension Bars	7	Steel Bar CG (a)	29.000
Max. Bar Size	8.0 mm	Concrete Cover (c)	29.0
Effective Concrete Area	58000.0 mm^2	Environmental Conditions	Interior: Normal

Crack Width (w) = 0.13 mm ≤ 0.4 mm **... Adequate** ✓

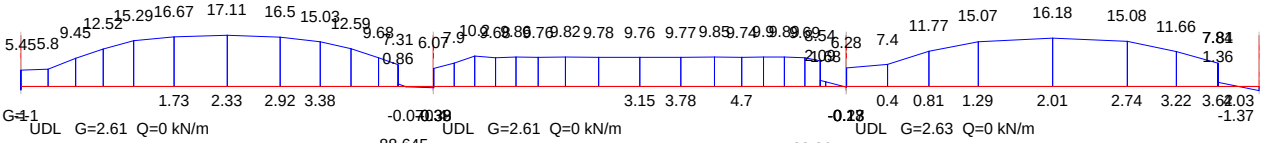
BEAM

Axis: 2 Storey: 1

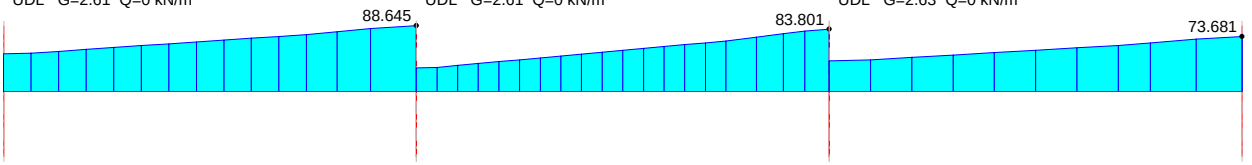
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B3 L=4650mm	1B2 L=6300mm	1B1 L=4025mm
Bw/H (mm)	250 / 500	250 / 500	250 / 500
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

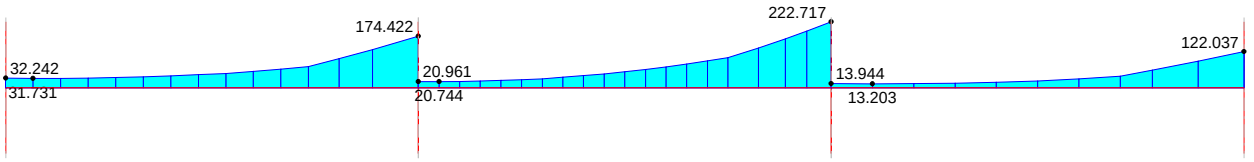
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	34.5	70.5	174.4	28.4	98.5	222.7	14.9	45.7	122.0
d (mm)	457.0	457.0	457.0	457.0	457.0	457.0	457.0	457.0	457.0
K/K'	0.13	0.26	0.65	0.11	0.37	0.83	0.06	0.17	0.45
x (mm)	57.13	57.34	156.01	57.13	81.95	210.96	57.13	57.13	103.65
Asm (mm2)	198.80	406.15	1105.07	163.36	580.46	1494.27	85.65	263.37	734.21
Asv (mm2)	165.62	238.89	184.46	170.36	209.83	164.64	154.16	197.37	134.08
As (mm2)	364.41	645.05	1105.07	333.72	790.29	1494.27	239.81	460.74	734.21
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	165.64	165.64	165.64	165.64	165.64	165.64	165.64	165.64	165.64

Bending (Bottom Edge) ...

M (kN.m)									
d (mm)									
K/K'									
x (mm)									
Asm (mm2)									
Asv (mm2)									
As (mm2)									
As' (mm2)									
As-min	159.47	165.64	159.47	159.47	165.64	159.47	159.47	165.64	159.47

Shear and Bending Design ...

Vd (kN)	53.0		59.0	54.5		52.7	49.3		42.9
v (N/mm2)	0.46		0.52	0.48		0.46	0.43		0.38
v-Rdc (N/mm2)	0.46	0.00	0.46	0.46	0.00	0.46	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	207.4		207.4	207.4		207.4	207.4		207.4
Vnom (kN)		92.9			121.7			92.9	
Td (kN.m)		9.9			0.8			7.5	
T-min (kN.m)		4.9			4.9			4.9	
t-RdC (N/mm2)		1.17						1.17	
Str. Ratio		2.348						1.900	
As-T (mm2)		518.22						393.88	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	10.18 < 38.37 OK	13.79 < 44.27 OK	8.81 < 38.37 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	X 603.19	X 603.19	603.19	X 603.19	X 603.19	603.19	603.19	X 603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

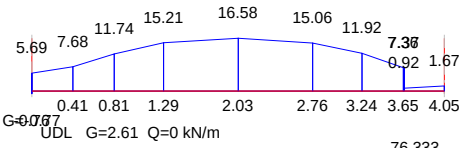
Top.Sup.	3T16		3T16	3T16		3T16	3T16		3T16
Top.Sup.									
Bot.Bars		3T16			3T16			3T16	
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: B Storey: 1

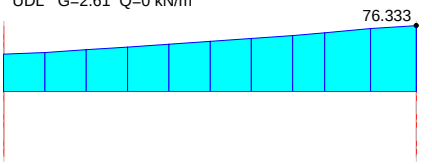
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B4	L=4050mm
Bw/H (mm)	250 / 500	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

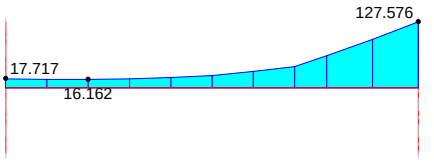
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	17.7	47.9	127.6
d (mm)	457.0	457.0	457.0
K/K'	0.07	0.18	0.47
x (mm)	57.13	57.13	108.91
Asm (mm2)	102.02	275.93	771.44
Asv (mm2)	157.33	205.53	147.43
As (mm2)	259.35	481.46	771.44
As' (mm2)	0.00	0.00	0.00
As-min	165.64	165.64	165.64

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	159.47	165.64	159.47

Shear and Bending Design ...

Vd (kN)	50.3		47.2
v (N/mm2)	0.44		0.41
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	207.4		207.4
Vnom (kN)		92.9	
Td (kN.m)		9.7	
T-min (kN.m)		4.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		2.111	
As-T (mm2)		509.33	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	8.86 < 29.52 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	X 603.19	X 603.19
Bot Edge	603.19	603.19	603.19

Steel Bars ...

Top.Sup.	3T16		3T16
Top.Sup.			
Bot.Bars		3T16	
Bot.Bars			
Bot.Sup.	3T16		3T16
Bot.Sup.			

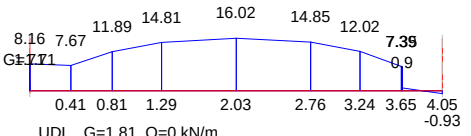
Side Bar

Axis: B Storey: 1 (/ 2)

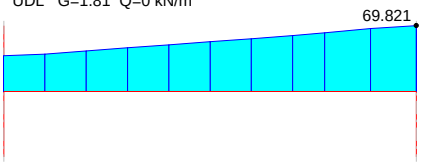
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B6	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

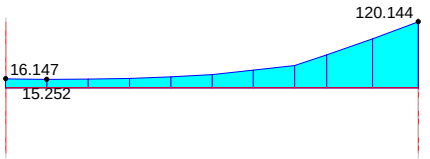
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	16.7	47.2	120.1
d (mm)	407.0	409.0	407.0
K/K'	0.10	0.27	0.70
x (mm)	50.88	53.70	153.34
Asm (mm2)	107.95	304.30	868.94
Asv (mm2)	142.66	188.28	139.32
As (mm2)	250.61	492.58	868.94
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	45.6		44.6
v (N/mm2)	0.56		0.55
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		4.9	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		2.758	
As-T (mm2)		316.95	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.9 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

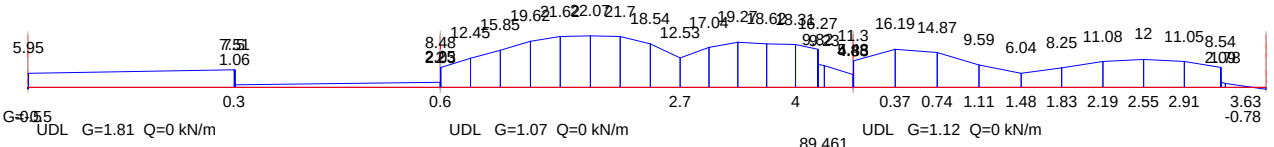
Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 3 Storey: 1

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B11 L=600mm	1B10 L=4650mm	1B9 L=3625mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

Loads



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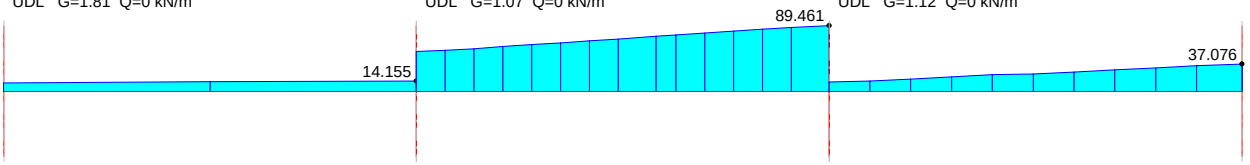

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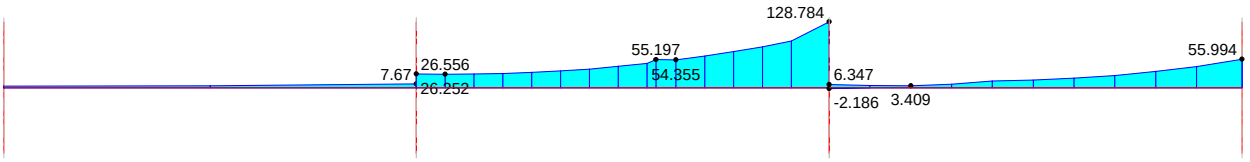


Bending (Top Edge) ...

M (kN.m)	3.2	3.9	7.7	29.7	65.8	128.8	6.3	26.2	56.0
d (mm)	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0
K/K'	0.02	0.02	0.04	0.17	0.38	0.75	0.04	0.15	0.33
x (mm)	50.88	50.88	50.88	50.88	77.15	167.01	50.88	50.88	64.83
Asm (mm2)	20.78	24.97	49.60	192.04	437.19	946.40	41.04	169.42	367.35
Asv (mm2)	28.97	34.84	25.26	234.64	251.35	241.67	95.76	94.18	55.72
As (mm2)	24.97	49.60	49.60	426.68	688.54	946.40	136.80	263.60	367.34
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	118.01	118.01	118.01	118.01	118.01	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)							2.2		
d (mm)							390.0		
K/K'							0.01		
x (mm)							48.75		
Asm (mm2)							14.75		
Asv (mm2)							95.76		
As (mm2)							14.75		
As' (mm2)							0.00		
As-min	0.00	118.01	113.08	113.08	118.01	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	15.9		16.7	75.1		77.3	30.6		17.8
v (N/mm2)	0.19		0.21	0.92		0.95	0.38		0.22
v-Rdc (N/mm2)	0.53	0.00	0.53	0.53	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	191.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			66.5	
Td (kN.m)		2.4			1.4			1.8	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	1.47 < 10.47 OK	11.43 < 39.28 OK	8.91 < 39.28 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19	603.19	X 603.19	X 603.19	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

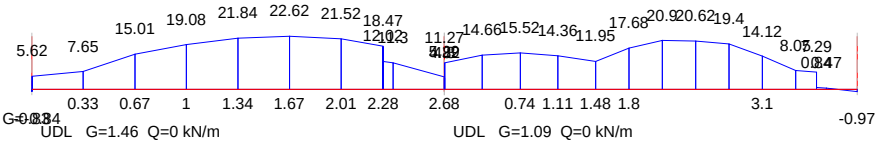
Top.Sup.			3T16	3T16		3T16	3T16		3T16
Top.Sup.									
Bot.Bars	3T16			3T16			3T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 3 Storey: 1 ...

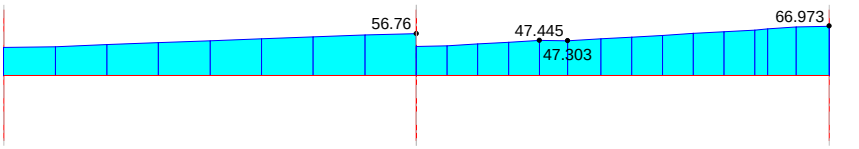
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B8 L=2675mm	1B7 L=4025mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

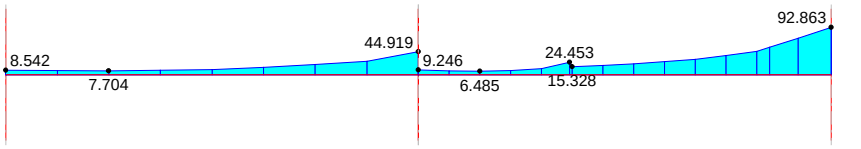
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	8.5	18.3	44.9	10.5	35.7	92.9
d (mm)	407.0	407.0	407.0	407.0	407.0	407.0
K/K'	0.05	0.11	0.26	0.06	0.21	0.54
x (mm)	50.88	50.88	51.28	50.88	50.88	113.27
Asm (mm2)	55.23	118.50	290.56	67.88	231.00	641.87
Asv (mm2)	114.36	161.57	148.14	94.87	183.26	115.20
As (mm2)	118.50	280.07	290.56	162.75	414.27	641.87
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.01	118.01	118.01	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)						
d (mm)						
K/K'						
x (mm)						
Asm (mm2)						
Asv (mm2)						
As (mm2)						
As' (mm2)						
As-min	113.08	118.01	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	36.6		47.4	30.4		36.9
v (N/mm2)	0.45		0.58	0.37		0.45
v-Rdc (N/mm2)	0.53	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5	
Td (kN.m)		3.4			1.4	
T-min (kN.m)		2.9			2.9	
t-RdC (N/mm2)		1.17				
Str. Ratio		1.690				
As-T (mm2)		219.14				
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.57 < 39.28 OK	9.89 < 34.04 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19	603.19	603.19	X 603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

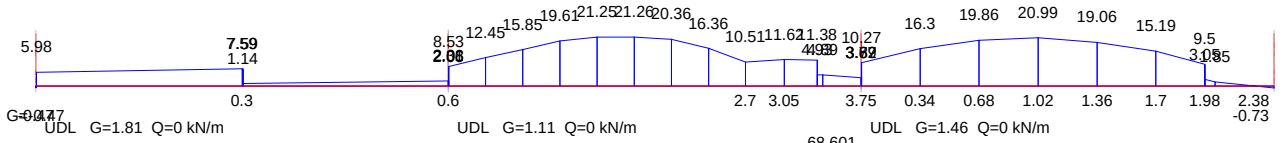
Top.Sup.	3T16		3T16	3T16		3T16
Top.Sup.						
Bot.Bars		3T16			3T16	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: 5 Storey: 1

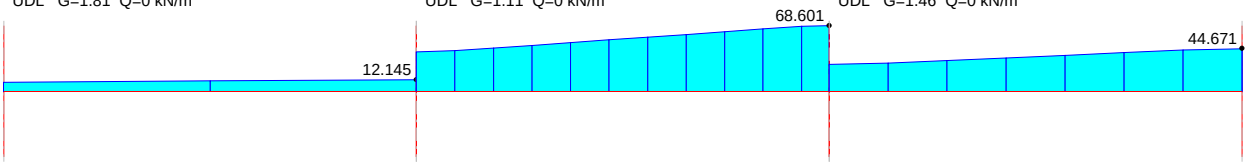
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B18 L=600mm	1B17 L=3750mm	1B16 L=2375mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

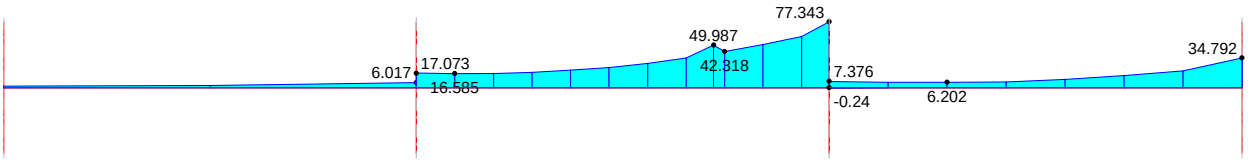
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	2.1	2.8	6.0	18.2	50.0	77.3	7.4	14.0	34.8
d (mm)	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0
K/K'	0.01	0.02	0.04	0.11	0.29	0.45	0.04	0.08	0.20
x (mm)	50.88	50.88	50.88	50.88	57.43	92.19	50.88	50.88	50.88
Asm (mm2)	13.89	17.86	38.91	117.59	325.41	522.41	47.69	90.35	224.96
Asv (mm2)	33.30	34.67	53.23	181.51	193.18	170.12	137.95	125.71	77.53
As (mm2)	17.86	38.91	38.91	299.10	518.60	522.41	90.35	216.06	224.96
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	118.01	118.01	118.01	118.01	118.01	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)							0.2		
d (mm)							390.0		
K/K'							0.00		
x (mm)							48.75		
Asm (mm2)							1.62		
Asv (mm2)							137.95		
As (mm2)							1.62		
As' (mm2)							0.00		
As-min	0.00	118.01	113.08	113.08	118.01	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	14.0		17.0	58.1		54.4	44.1		24.8
v (N/mm2)	0.17		0.21	0.71		0.67	0.54		0.30
v-Rdc (N/mm2)	0.66	0.00	0.53	0.53	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	191.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			108.8	
Td (kN.m)		2.2			0.9			0.0	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	1.47 < 10.47 OK	9.21 < 39.28 OK	5.84 < 39.28 OK

Supplied Steel Areas (mm2)

Top Edge	1206.37	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

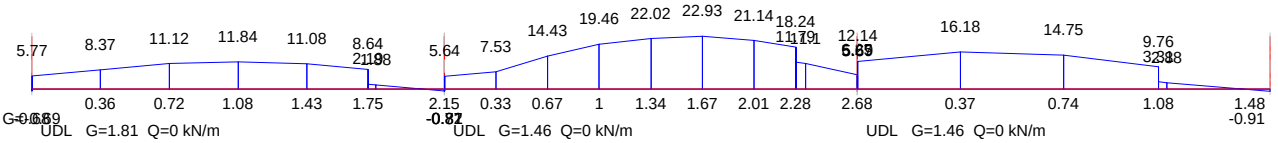
Top.Sup.	3T16		3T16	3T16		3T16	3T16		3T16
Top.Sup.									
Bot.Bars	3T16			3T16			3T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 5 Storey: 1 ...

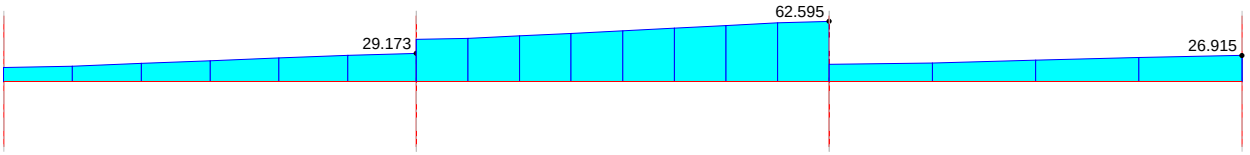
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B15 L=2150mm	1B14 L=2675mm	1B13 L=1475mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

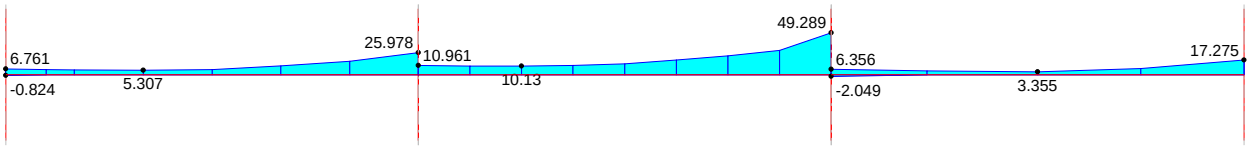
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	6.8	11.6	26.0	11.0	20.7	49.3	6.4	6.4	17.3
d (mm)	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0
K/K'	0.04	0.07	0.15	0.06	0.12	0.29	0.04	0.04	0.10
x (mm)	50.88	50.88	50.88	50.88	50.88	56.57	50.88	50.88	50.88
Asm (mm2)	43.72	74.83	167.97	70.87	134.16	320.58	41.10	41.40	111.70
Asv (mm2)	62.31	78.18	56.53	108.37	179.81	154.21	73.17	76.08	28.97
As (mm2)	74.83	153.01	167.97	134.16	313.97	320.58	41.40	111.70	111.70
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.01	118.01	118.01	118.01	118.01	118.01	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)	0.8						2.0		
d (mm)	390.0						390.0		
K/K'	0.01						0.01		
x (mm)	48.75						48.75		
Asm (mm2)	5.56						13.83		
Asv (mm2)	62.31						73.17		
As (mm2)	5.56						13.83		
As' (mm2)	0.00						0.00		
As-min	113.08	118.01	113.08	113.08	118.01	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	19.9		18.1	34.7		49.3	23.4		15.9
v (N/mm2)	0.24		0.22	0.43		0.61	0.29		0.19
v-Rdc (N/mm2)	0.53	0.00	0.53	0.53	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			108.8	
Td (kN.m)		4.7			1.6			0.3	
T-min (kN.m)		2.9			2.9			2.9	
t-RdC (N/mm2)		1.17							
Str. Ratio		1.232							
As-T (mm2)		300.46							
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.28 < 39.28 OK	6.57 < 39.28 OK	3.62 < 39.28 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

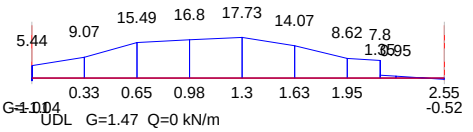
Top.Sup.	3T16		3T16	3T16		3T16	3T16		3T16
Top.Sup.									
Bot.Bars		3T16			3T16			3T16	
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 5 Storey: 1 ...

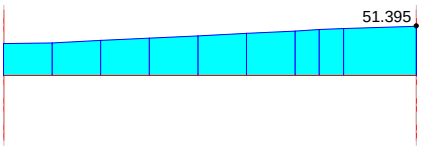
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B12	L=2550mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

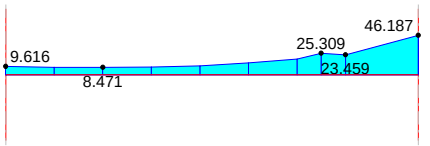
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	9.6	18.9	46.2
d (mm)	407.0	407.0	407.0
K/K'	0.06	0.11	0.27
x (mm)	50.88	50.88	52.81
Asm (mm2)	62.17	122.03	299.23
Asv (mm2)	91.77	144.41	112.08
As (mm2)	122.03	266.44	299.23
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	29.4		35.9
v (N/mm2)	0.36		0.44
v-Rdc (N/mm2)	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		1.2	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.27 < 34.04 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19

Steel Bars ...

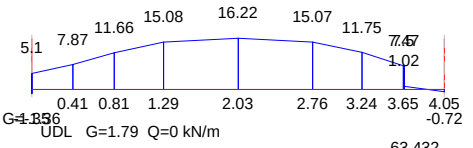
Top.Sup.	3T16		3T16
Top.Sup.			
Bot.Bars		3T16	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: M Storey: 1

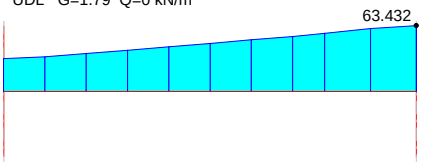
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B19	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

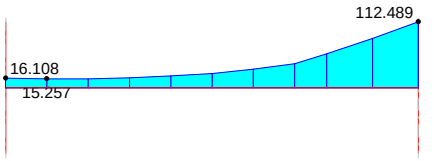
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	16.8	46.6	112.5
d (mm)	407.0	407.0	407.0
K/K'	0.10	0.27	0.66
x (mm)	50.88	53.31	141.66
Asm (mm2)	108.47	302.10	802.72
Asv (mm2)	140.51	168.40	138.94
As (mm2)	248.98	470.50	802.72
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	45.0		44.5
v (N/mm2)	0.55		0.55
v-Rdc (N/mm2)	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		3.1	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.802	
As-T (mm2)		200.23	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.95 < 26.19 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	X 603.19
Bot Edge	603.19	603.19	603.19

Steel Bars ...

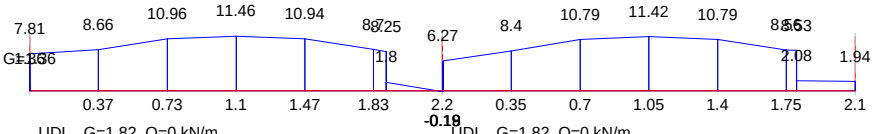
Top.Sup.	3T16		3T16
Top.Sup.			
Bot.Bars		3T16	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: L Storey: 1

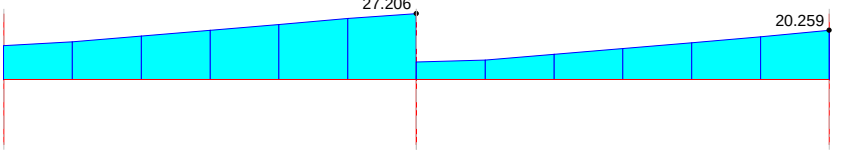
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B20 L=2200mm	1B21 L=2100mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

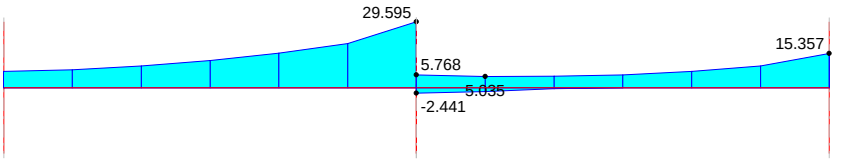
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	9.0	16.3	29.6	5.8	8.5	15.4
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.05	0.09	0.17	0.03	0.05	0.09
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	58.12	104.73	190.42	37.11	54.40	98.81
Asv (mm2)	35.45	72.31	76.89	76.59	28.17	35.16
As (mm2)	93.57	177.04	190.42	54.40	82.57	98.81
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				2.4	0.7	
d (mm)				390.0	409.0	
K/K'				0.02	0.00	
x (mm)				48.75	51.13	
Asm (mm2)				16.47	4.35	
Asv (mm2)				76.59	28.17	
As (mm2)				16.47	16.47	
As' (mm2)				0.00	0.00	
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	12.9		24.6	24.5		11.3
v (N/mm2)	0.16		0.30	0.30		0.14
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			108.8	
Td (kN.m)		1.0			0.5	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.38 < 76.7 OK	5.13 < 76.7 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T12		2T12	2T12		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

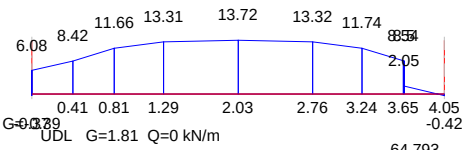
Axis: M **Storey: 1** (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

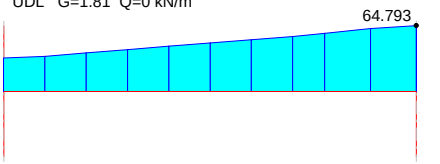
1B22 L=4050mm

Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

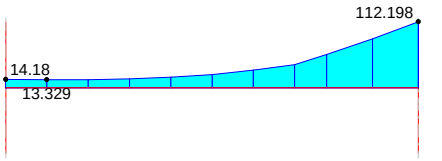
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	14.8	44.8	112.2
d (mm)	409.0	409.0	409.0
K/K'	0.09	0.26	0.65
x (mm)	51.13	51.13	140.27
Asm (mm2)	95.54	288.37	794.85
Asv (mm2)	130.45	172.56	129.68
As (mm2)	225.99	460.92	794.85
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	41.7		41.5
v (N/mm2)	0.51		0.51
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		2.6	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 9.9 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	X 226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

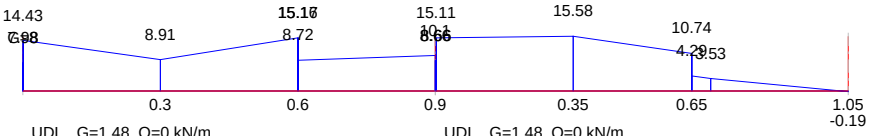
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

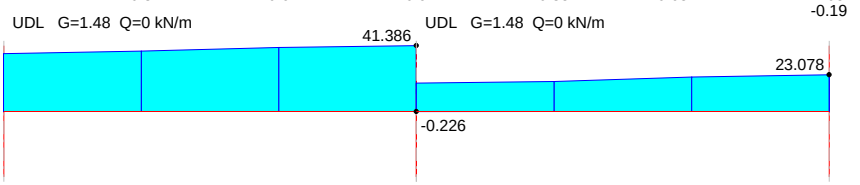
Axis: 4 Storey: 1

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B24 L=900mm	1B23 L=1050mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

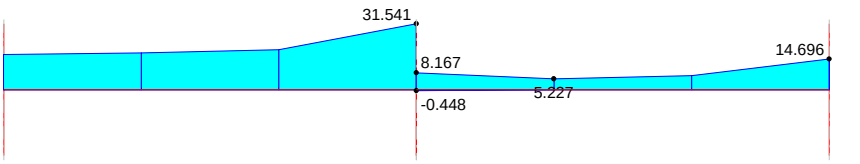
Loads





Shear Force (Envelope)

Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	17.5	18.9	31.5	8.2	6.6	14.7
d (mm)	390.0	390.0	399.0	399.0	409.0	409.0
K/K'	0.11	0.12	0.19	0.05	0.04	0.08
x (mm)	48.75	48.75	49.88	49.88	51.13	51.13
Asm (mm2)	118.28	127.86	208.02	53.87	42.58	94.56
Asv (mm2)	145.49	124.68	187.52	90.78	67.14	31.75
As (mm2)	127.86	208.02	208.02	53.87	94.56	94.56
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	113.08	115.69	115.69	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				0.4		
d (mm)				390.0		
K/K'				0.00		
x (mm)				48.75		
Asm (mm2)				3.03		
Asv (mm2)				90.78		
As (mm2)				3.03		
As' (mm2)				0.00		
As-min	0.00	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	46.6		60.0	29.0		14.9
v (N/mm2)	0.60		0.75	0.36		0.18
v-Rdc (N/mm2)	0.75	0.00	0.73	0.73	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	192.8		183.8	183.8		183.8
Vnom (kN)		108.8			108.8	
Td (kN.m)		0.1			0.0	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	2.26 < 8.39 OK			2.57 < 76.7 OK		

Supplied Steel Areas (mm2)

Top Edge	1963.50	1963.50	1608.49	1608.49	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

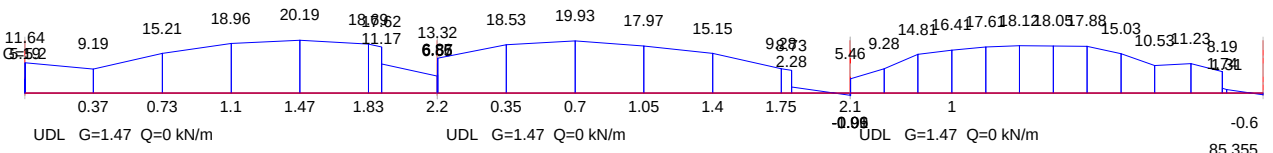
Top.Sup.			2T32	2T32		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: J Storey: 1

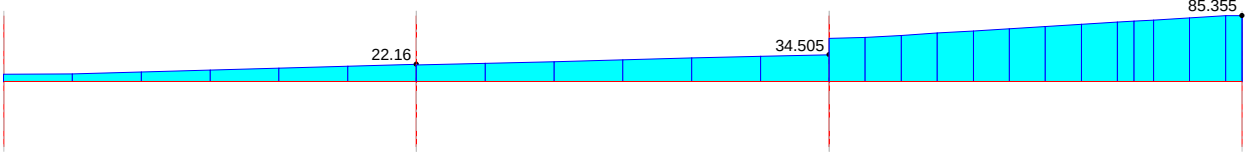
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B25 L=2200mm	1B26 L=2100mm	1B28 L=4050mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

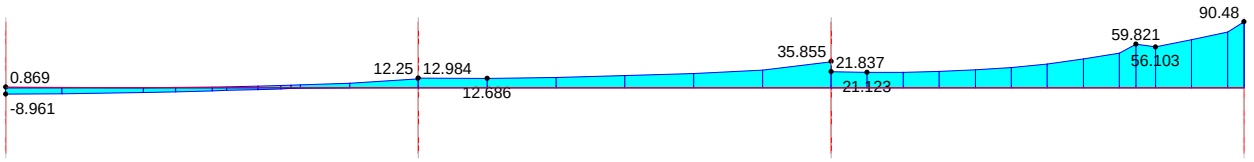
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	1.7	4.4	12.3	13.4	20.4	35.9	22.7	55.8	90.5
d (mm)	409.0	409.0	409.0	409.0	409.0	407.0	407.0	409.0	407.0
K/K'	0.01	0.03	0.07	0.08	0.12	0.21	0.13	0.32	0.53
x (mm)	51.13	51.13	51.13	51.13	51.13	50.88	50.88	64.20	109.96
Asm (mm2)	11.22	28.22	78.82	86.36	131.41	231.83	146.77	363.79	623.11
Asv (mm2)	126.91	56.52	76.89	59.79	95.95	123.37	175.33	243.51	168.25
As (mm2)	28.22	78.82	78.82	131.41	227.35	231.83	322.09	607.29	623.11
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.01	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)	9.0	7.0	0.0						
d (mm)	390.0	409.0	390.0						
K/K'	0.06	0.04	0.00						
x (mm)	48.75	51.13	48.75						
Asm (mm2)	60.47	44.89	0.16						
Asv (mm2)	126.91	56.52	76.89						
As (mm2)	60.47	60.47	44.89						
As' (mm2)	0.00	0.00	0.00						
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	40.6		24.6	19.1		39.5	56.1		53.8
v (N/mm2)	0.50		0.30	0.23		0.48	0.69		0.66
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.46	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		108.8			66.5			66.5	
Td (kN.m)		0.6			1.7			1.5	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.38 < 76.7 OK	5.13 < 88.5 OK	9.9 < 70.5 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	X 226.19	402.12	402.12	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T12		2T12	2T12		2T16	2T16		2T16
Top.Sup.									
Bot.Bars	2T12			2T12			2T12		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 6 Storey: 1

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

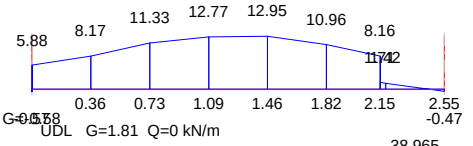
1B27 L=2550mm

Bw/H (mm) 200 / 450

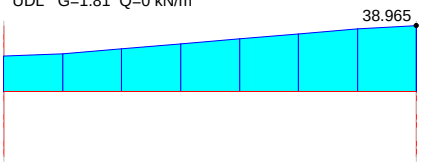
Flange Bf/Hf ---

C25/30 / Grade 460 (Type 2)

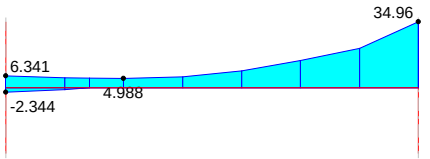
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	6.3	14.3	35.0
d (mm)	409.0	409.0	409.0
K/K'	0.04	0.08	0.20
x (mm)	51.13	51.13	51.13
Asm (mm2)	40.80	92.15	224.94
Asv (mm2)	74.79	106.14	73.37
As (mm2)	92.15	198.29	224.94
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)	2.3		
d (mm)	390.0		
K/K'	0.01		
x (mm)	48.75		
Asm (mm2)	15.82		
Asv (mm2)	74.79		
As (mm2)	15.82		
As' (mm2)	0.00		
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	23.9		23.5
v (N/mm2)	0.29		0.29
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		3.4	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.621	
As-T (mm2)		219.07	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 6.23 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	X 226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

Top.Sup. 2T12 2T12

Top.Sup.

Bot.Bars 2T12

Bot.Bars

Bot.Sup.

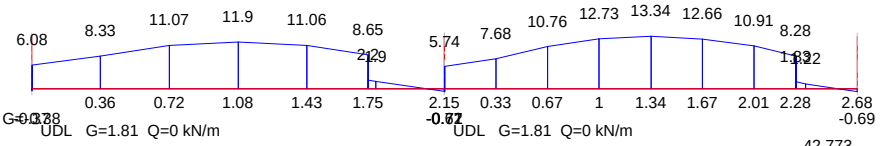
Side Bar

Axis: 7 Storey: 1

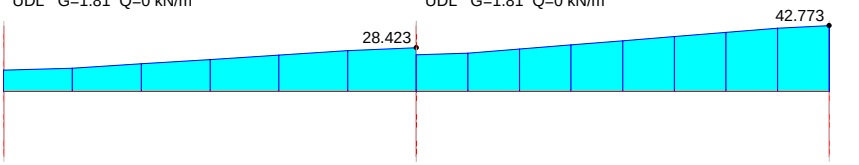
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B30 L=2150mm	1B29 L=2675mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

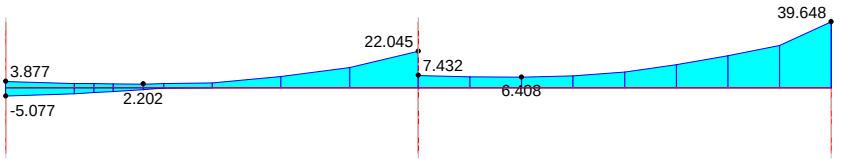
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	3.9	7.9	22.0	7.4	17.6	39.6
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.02	0.05	0.13	0.04	0.10	0.23
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	24.94	50.59	141.84	47.82	113.49	255.10
Asv (mm2)	66.47	75.83	54.90	78.34	117.40	76.53
As (mm2)	50.59	126.43	141.84	113.49	230.89	255.10
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)	5.1	1.7				
d (mm)	390.0	409.0				
K/K'	0.03	0.01				
x (mm)	48.75	51.13				
Asm (mm2)	34.26	11.25				
Asv (mm2)	66.47	75.83				
As (mm2)	34.26	34.26				
As' (mm2)	0.00	0.00				
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	21.3		17.6	25.1		24.5
v (N/mm2)	0.26		0.21	0.31		0.30
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5	
Td (kN.m)		3.9			5.7	
T-min (kN.m)		2.9			2.9	
t-RdC (N/mm2)		1.17			1.17	
Str. Ratio		1.359			2.018	
As-T (mm2)		251.33			363.71	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.26 < 76.7 OK	6.54 < 76.7 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	X 226.19	X 226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

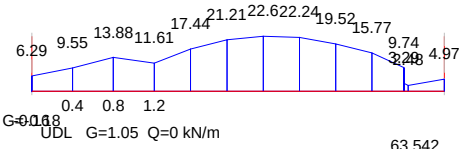
Top.Sup.	2T12		2T12	2T12		2T12
Top.Sup.						
Bot.Bars	2T12			2T12		
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: I Storey: 1

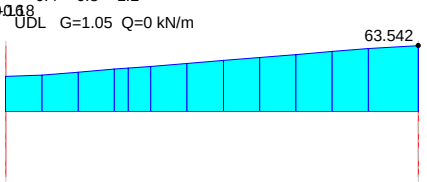
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B31	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

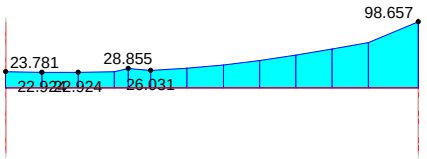
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	25.6	51.2	98.7
d (mm)	407.0	409.0	407.0
K/K'	0.15	0.30	0.58
x (mm)	50.88	58.54	121.43
Asm (mm2)	165.31	331.74	688.13
Asv (mm2)	208.03	173.98	205.64
As (mm2)	331.74	505.71	688.13
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	66.6		65.8
v (N/mm2)	0.82		0.81
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		0.8	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.9 < 42.16 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

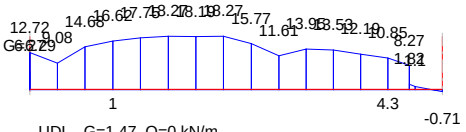
Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: I Storey: 1 (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

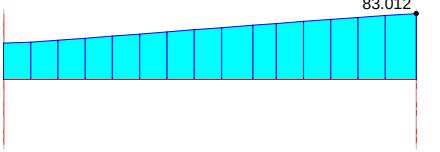
	1B32	L=4950mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

Loads

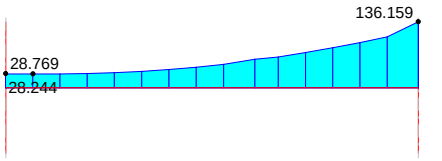


UDL G=1.47 Q=0 kN/m

Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	31.6	73.3	136.2
d (mm)	405.0	409.0	407.0
K/K'	0.19	0.42	0.80
x (mm)	50.63	86.38	179.13
Asm (mm2)	205.30	489.46	1015.06
Asv (mm2)	223.73	228.88	197.80
As (mm2)	429.03	718.33	1015.06
As' (mm2)	0.00	0.00	0.00
As-min	117.43	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	71.6		63.3
v (N/mm2)	0.88		0.78
v-Rdc (N/mm2)	0.53	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		3.8	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		2.589	
As-T (mm2)		242.80	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	12.16 < 30.35 OK

Supplied Steel Areas (mm2)

Top Edge	628.32	X 226.19	X 402.12
Bot Edge	402.12	402.12	402.12

Steel Bars ...

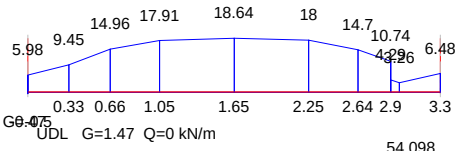
Top.Sup.	2T20		2T16
Top.Sup.			
Bot.Bars		2T16	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 4 Storey: 1 (/ 2)

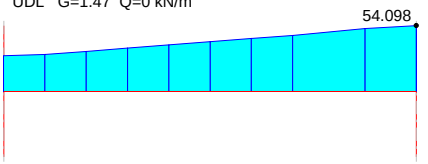
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B33	L=3300mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

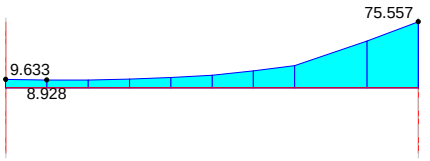
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	9.6	28.4	75.6
d (mm)	409.0	409.0	409.0
K/K'	0.06	0.16	0.44
x (mm)	51.13	51.13	89.30
Asm (mm2)	62.07	182.98	506.03
Asv (mm2)	144.85	146.45	136.73
As (mm2)	182.98	329.43	506.03
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	46.4		43.7
v (N/mm2)	0.57		0.53
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		108.8	
Td (kN.m)		0.4	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 8.07 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	X 226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

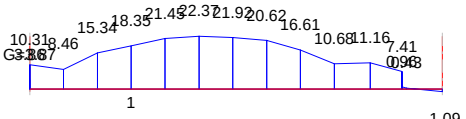
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: D Storey: 1

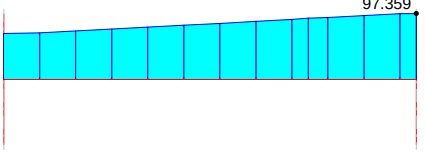
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B34	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

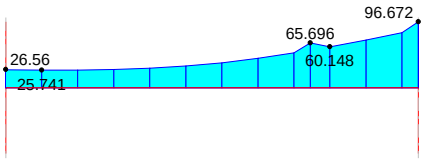
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	27.1	61.0	96.7
d (mm)	407.0	409.0	407.0
K/K'	0.16	0.35	0.56
x (mm)	50.88	70.72	118.62
Asm (mm2)	175.15	400.76	672.17
Asv (mm2)	203.68	281.02	213.44
As (mm2)	378.82	672.17	672.17
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	65.2		68.3
v (N/mm2)	0.80		0.84
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		108.8	
Td (kN.m)		0.1	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 9.9 < 42.01 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

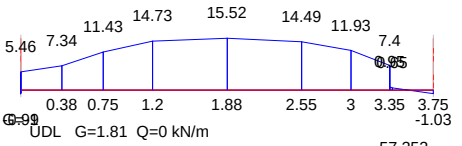
Axis: 6 Storey: 1 (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

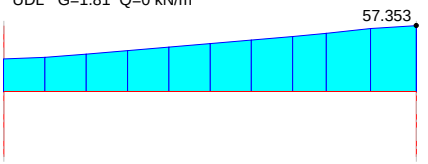
1B35 L=3750mm

Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

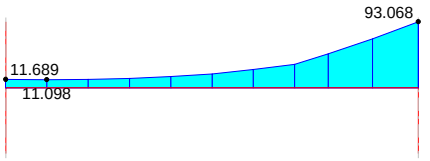
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	12.6	37.5	93.1
d (mm)	409.0	409.0	409.0
K/K'	0.07	0.22	0.54
x (mm)	51.13	51.13	112.84
Asm (mm2)	81.28	241.60	639.44
Asv (mm2)	126.82	151.74	119.87
As (mm2)	208.11	393.34	639.44
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	40.6		38.4
v (N/mm2)	0.50		0.47
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		4.4	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		2.309	
As-T (mm2)		282.70	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.17 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	X 226.19	X 226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

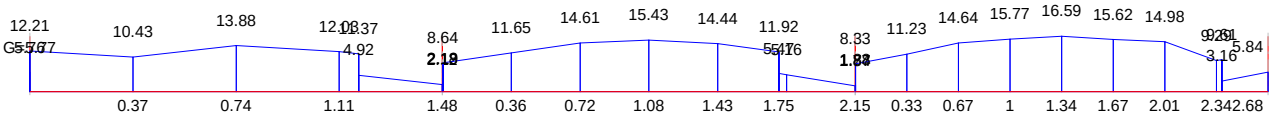
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 2a Storey: 1

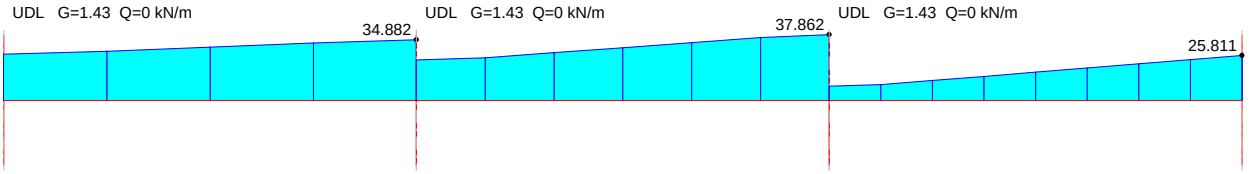
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B41 L=1475mm	1B40 L=2150mm	1B39 L=2675mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

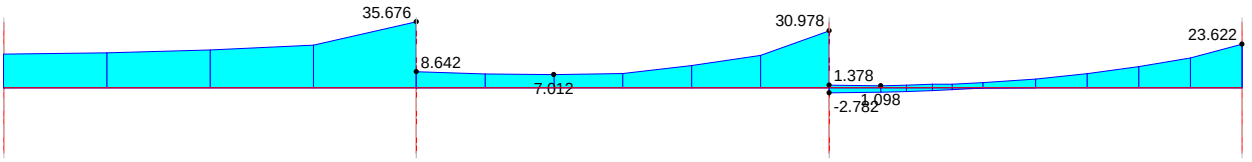
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	19.0	22.2	35.7	8.6	13.4	31.0	1.9	11.2	23.6
d (mm)	390.0	390.0	399.0	399.0	409.0	407.0	407.0	409.0	409.0
K/K'	0.12	0.14	0.22	0.05	0.08	0.18	0.01	0.06	0.14
x (mm)	48.75	48.75	49.88	49.88	51.13	50.88	50.88	51.13	51.13
Asm (mm2)	128.40	149.57	235.30	57.00	85.97	200.30	12.25	72.07	151.99
Asv (mm2)	57.11	100.78	117.21	85.05	105.87	77.40	138.04	65.14	60.99
As (mm2)	149.57	235.30	235.30	85.97	191.83	200.30	72.07	137.21	151.99
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	113.08	115.69	115.69	118.59	118.01	118.01	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)							2.8	1.0	
d (mm)							390.0	409.0	
K/K'							0.02	0.01	
x (mm)							48.75	51.13	
Asm (mm2)							18.77	6.74	
Asv (mm2)							138.04	65.14	
As (mm2)							18.77	18.77	
As' (mm2)							0.00	0.00	
As-min	0.00	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	18.3		37.5	27.2		24.8	44.2		19.5
v (N/mm2)	0.23		0.47	0.34		0.30	0.54		0.24
v-Rdc (N/mm2)	0.75	0.00	0.73	0.73	0.00	0.46	0.46	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	192.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			66.5	
Td (kN.m)		0.9			2.2			0.9	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	3.7 < 8.39 OK	5.26 < 88.5 OK	6.54 < 76.7 OK

Supplied Steel Areas (mm2)

Top Edge	1963.50	1963.50	1608.49	1608.49	226.19	402.12	402.12	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

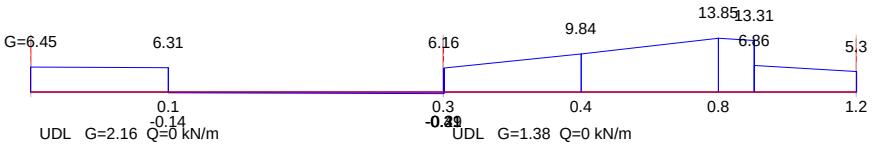
Top.Sup.			2T32	2T32		2T16	2T16		2T12
Top.Sup.									
Bot.Bars	2T12			2T12			2T12		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: E Storey: 1

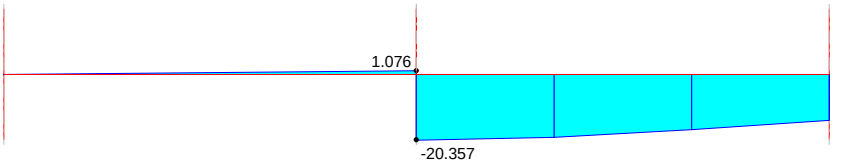
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B42 L=300mm	1B43 L=1203.6mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

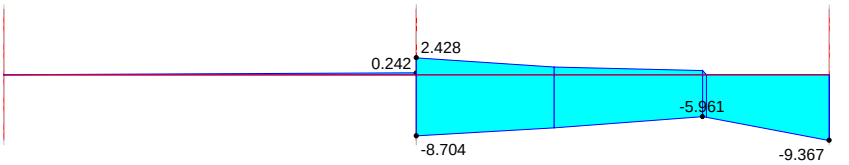
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	0.0	0.1	0.2	2.4	1.1	1.9
d (mm)	390.0	390.0	399.0	399.0	409.0	409.0
K/K'	0.00	0.00	0.00	0.01	0.01	0.01
x (mm)	48.75	48.75	49.88	49.88	51.13	51.13
Asm (mm2)	0.14	0.41	1.60	16.01	6.77	12.02
Asv (mm2)	0.31	0.84	0.73	141.33	60.69	103.12
As (mm2)	0.41	1.25	1.60	16.01	16.01	12.02
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	113.08	115.69	115.69	118.59	0.00

Bending (Bottom Edge) ...

M (kN.m)				8.7	7.5	7.2
d (mm)				390.0	409.0	409.0
K/K'				0.06	0.04	0.04
x (mm)				48.75	51.13	51.13
Asm (mm2)				58.73	48.07	46.30
Asv (mm2)				141.33	60.69	103.12
As (mm2)				58.73	58.73	48.07
As' (mm2)				0.00	0.00	0.00
As-min	0.00	118.59	113.08	113.08	118.59	0.00

Shear and Bending Design ...

Vd (kN)	0.1		0.2	45.2		33.0
v (N/mm2)	0.00		0.00	0.57		0.40
v-Rdc (N/mm2)	0.75	0.00	0.58	0.58	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	192.8		183.8	183.8		192.8
Vnom (kN)		108.8			66.5	
Td (kN.m)		0.0			2.2	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links			**R10-300**			**R10-300**

Deflection Check ...

L/d	0.75 < 9.6 OK			3.02 < 9.6 OK		

Supplied Steel Areas (mm2)

Top Edge	1963.50	1963.50	804.25	804.25	X 0.00	X 0.00
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

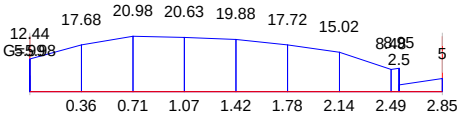
Top.Sup.			1T32	1T32		
Top.Sup.						
Bot.Bars	2T12			2T12		
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: E Storey: 1 (/ 2)

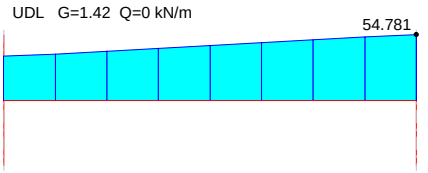
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B44	L=2846.4mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

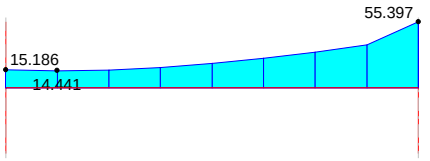
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	15.2	28.2	55.4
d (mm)	409.0	409.0	407.0
K/K'	0.09	0.16	0.32
x (mm)	51.13	51.13	64.09
Asm (mm2)	97.71	181.34	363.15
Asv (mm2)	104.68	155.38	172.66
As (mm2)	181.34	336.72	363.15
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	33.5		55.2
v (N/mm2)	0.41		0.68
v-Rdc (N/mm2)	0.38	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		4.1	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		2.186	
As-T (mm2)		260.24	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.96 < 53.21 OK

Supplied Steel Areas (mm2)

Top Edge	X 226.19	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

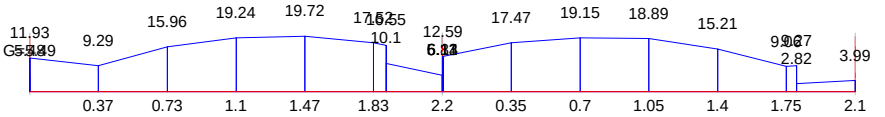
Top.Sup.	2T12		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: C Storey: 1

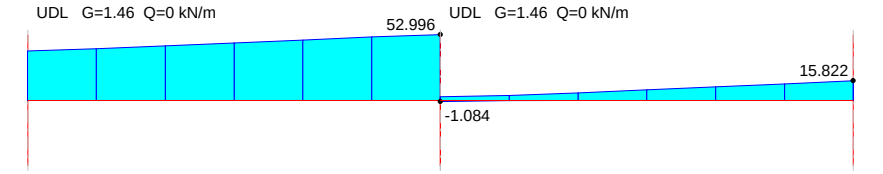
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B45 L=2200mm	1B46 L=2100mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

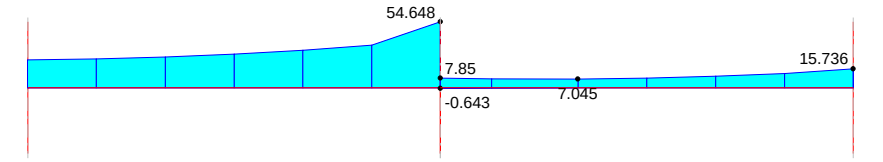
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	24.7	31.9	54.6	7.9	10.5	15.7
d (mm)	407.0	407.0	407.0	407.0	409.0	409.0
K/K'	0.14	0.19	0.32	0.05	0.06	0.09
x (mm)	50.88	50.88	63.16	50.88	51.13	51.13
Asm (mm2)	159.94	206.12	357.89	50.76	67.54	101.25
Asv (mm2)	35.93	153.42	174.02	151.30	36.68	34.10
As (mm2)	195.87	357.89	357.89	67.54	101.25	101.25
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.01	118.01	118.01	118.01	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				0.6		
d (mm)				390.0		
K/K'				0.00		
x (mm)				48.75		
Asm (mm2)				4.34		
Asv (mm2)				151.30		
As (mm2)				4.34		
As' (mm2)				0.00		
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	12.3		55.7	48.4		10.9
v (N/mm2)	0.15		0.68	0.59		0.13
v-Rdc (N/mm2)	0.46	0.00	0.46	0.46	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5	
Td (kN.m)		1.1			1.2	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d		5.38 < 76.7 OK			5.13 < 76.7 OK	

Supplied Steel Areas (mm2)

Top Edge	402.12	402.12	402.12	402.12	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

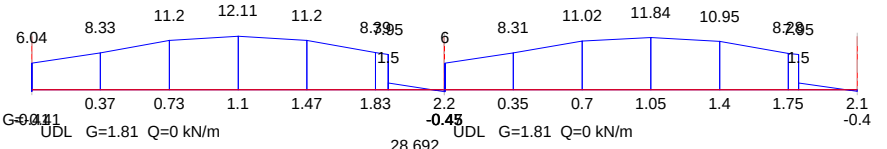
Top.Sup.			2T16	2T16		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: A Storey: 1

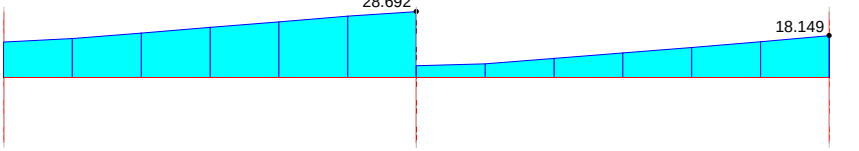
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B47 L=2200mm	1B48 L=2100mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

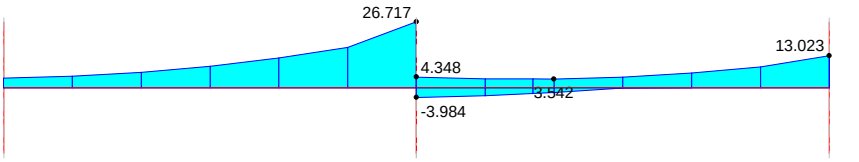
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	5.5	12.8	26.7	4.3	7.0	13.0
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.03	0.07	0.15	0.03	0.04	0.08
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	35.52	82.58	171.90	27.97	44.99	83.79
Asv (mm2)	32.67	76.89	71.37	70.82	33.50	35.74
As (mm2)	68.18	159.47	171.90	44.99	78.49	83.79
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				4.0	2.2	
d (mm)				390.0	409.0	
K/K'				0.03	0.01	
x (mm)				48.75	51.13	
Asm (mm2)				26.88	14.09	
Asv (mm2)				70.82	33.50	
As (mm2)				26.88	26.88	
As' (mm2)				0.00	0.00	
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	14.5		22.8	22.7		12.7
v (N/mm2)	0.18		0.28	0.28		0.15
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			108.8	
Td (kN.m)		2.1			0.1	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.38 < 76.7 OK			5.13 < 76.7 OK		

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

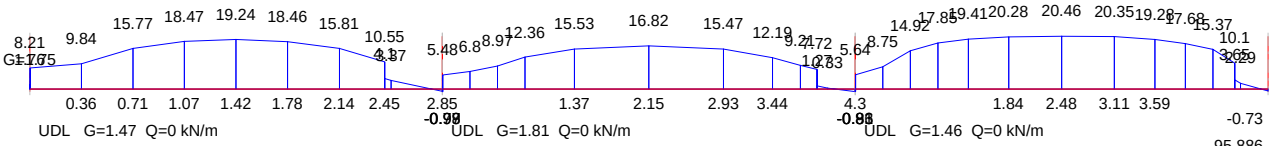
Top.Sup.	2T12		2T12	2T12		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: H Storey: 1

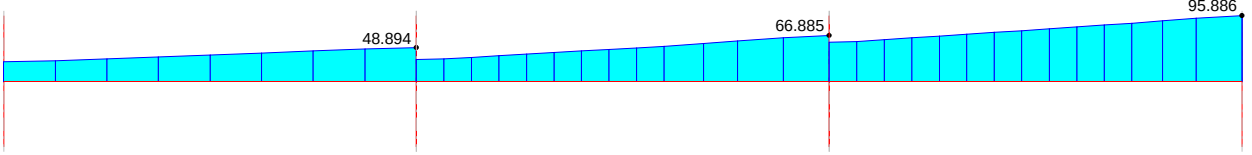
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B49 L=2846.4mm	1B37 L=4300mm	1B51 L=4950mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

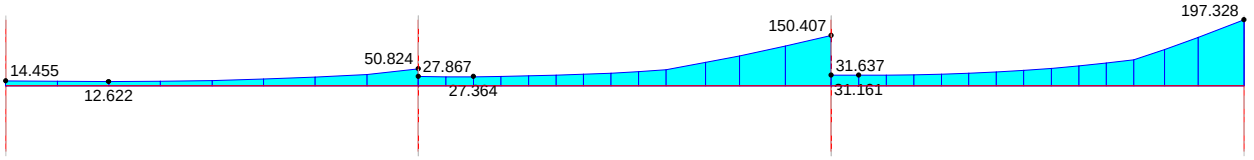
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	14.5	24.3	50.8	29.5	77.2	150.4	34.7	76.7	197.3
d (mm)	409.0	409.0	407.0	407.0	409.0	405.0	405.0	409.0	405.0
K/K'	0.08	0.14	0.30	0.17	0.45	0.89	0.20	0.44	1.16
x (mm)	51.13	51.13	58.45	50.88	91.43	205.58	50.63	90.82	243.00
Asm (mm2)	93.01	156.27	331.21	191.01	518.11	1164.98	225.17	514.66	1578.39
Asv (mm2)	118.67	135.78	124.58	146.88	177.08	155.74	240.60	264.38	231.76
As (mm2)	156.27	292.05	331.21	337.89	695.20	1164.98	465.77	779.03	1578.39
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	201.39
As-min	118.59	118.59	118.01	118.01	118.59	117.43	117.43	118.59	117.43

Bending (Bottom Edge) ...

M (kN.m)									
d (mm)									
K/K'									
x (mm)									
Asm (mm2)									
Asv (mm2)									
As (mm2)									
As' (mm2)									
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	38.0		39.9	47.0		49.8	77.0		74.2
v (N/mm2)	0.46		0.49	0.58		0.62	0.95		0.92
v-Rdc (N/mm2)	0.38	0.00	0.46	0.46	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		108.8			66.5			108.8	
Td (kN.m)		0.3			2.8			0.6	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.96 < 76.7 OK	10.51 < 88.5 OK	12.16 < 38.6 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	X 226.19	402.12	402.12	X 226.19	X 628.32	628.32	X 226.19	X 628.32
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	402.12	402.12	402.12

Steel Bars ...

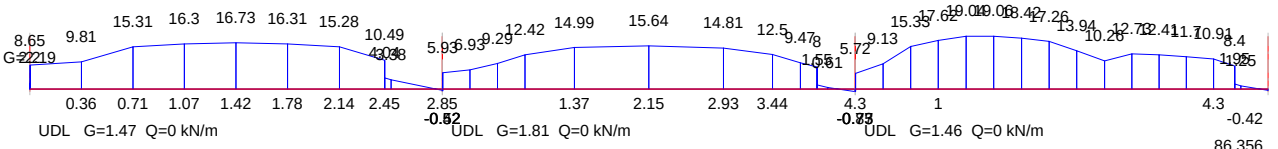
Top.Sup.	2T12		2T16	2T16		2T20	2T20		2T20
Top.Sup.									
Bot.Bars	2T12			2T12			2T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: F Storey: 1

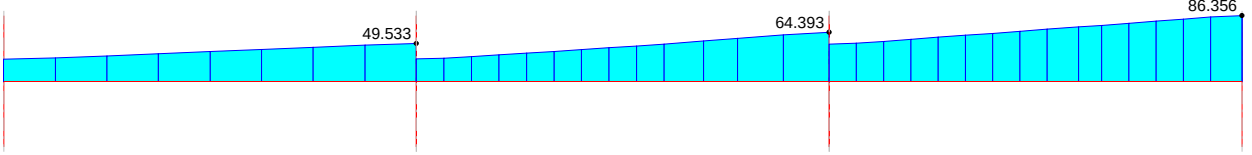
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B50 L=2846.4mm	1B38 L=4300mm	1B36 L=4950mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

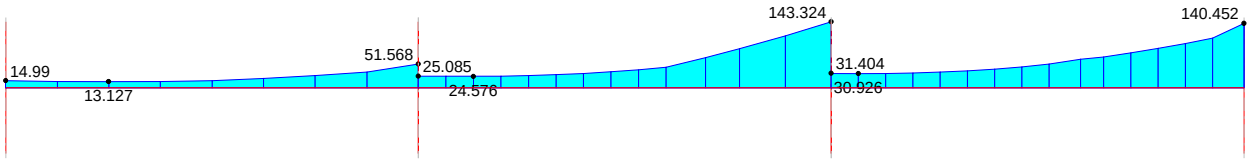
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	15.0	24.8	51.6	26.7	73.0	143.3	34.4	76.5	140.5
d (mm)	409.0	409.0	407.0	407.0	409.0	405.0	405.0	409.0	405.0
K/K'	0.09	0.14	0.30	0.16	0.42	0.85	0.20	0.44	0.83
x (mm)	51.13	51.13	59.36	50.88	86.01	192.86	50.63	90.52	187.85
Asm (mm2)	96.45	159.48	336.38	172.95	487.39	1092.89	223.66	512.93	1064.48
Asv (mm2)	108.65	137.77	117.87	143.95	169.29	151.23	233.06	239.33	204.24
As (mm2)	159.48	297.24	336.38	316.90	656.69	1092.90	456.72	752.25	1064.48
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.01	118.01	118.59	117.43	117.43	118.59	117.43

Bending (Bottom Edge) ...

M (kN.m)									
d (mm)									
K/K'									
x (mm)									
Asm (mm2)									
Asv (mm2)									
As (mm2)									
As' (mm2)									
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	34.8		37.7	46.1		48.4	74.6		65.4
v (N/mm2)	0.43		0.46	0.57		0.60	0.92		0.81
v-Rdc (N/mm2)	0.38	0.00	0.46	0.46	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		108.8			66.5			66.5	
Td (kN.m)		0.2			2.1			2.4	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.96 < 76.7 OK	10.51 < 88.5 OK	12.16 < 39.45 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	X 226.19	402.12	402.12	X 226.19	X 628.32	628.32	X 226.19	X 628.32
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	402.12	402.12	402.12

Steel Bars ...

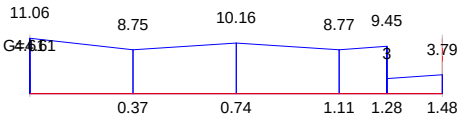
Top.Sup.	2T12		2T16	2T16		2T20	2T20		2T20
Top.Sup.									
Bot.Bars	2T12			2T12			2T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 6a Storey: 1

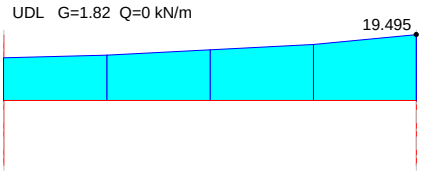
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B52	L=1475mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

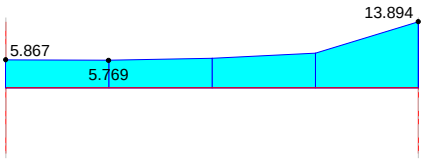
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	5.9	7.1	13.9
d (mm)	409.0	409.0	409.0
K/K'	0.03	0.04	0.08
x (mm)	51.13	51.13	51.13
Asm (mm2)	37.75	45.85	89.39
Asv (mm2)	35.36	27.38	13.63
As (mm2)	45.85	73.23	89.39
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	11.3		4.4
v (N/mm2)	0.14		0.05
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		2.0	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	3.61 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

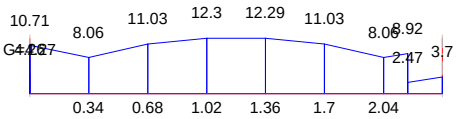
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 6a Storey: 1 (/ 2)

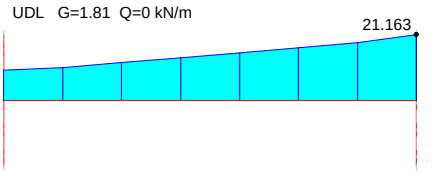
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	1B53	L=2375mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

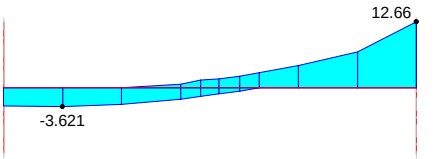
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	0.8	4.5	12.7
d (mm)	409.0	409.0	409.0
K/K'	0.00	0.03	0.07
x (mm)	51.13	51.13	51.13
Asm (mm2)	5.20	29.27	81.45
Asv (mm2)	65.96	53.45	65.11
As (mm2)	29.27	81.45	81.45
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)	3.6	3.2	
d (mm)	390.0	409.0	
K/K'	0.02	0.02	
x (mm)	48.75	51.13	
Asm (mm2)	24.43	20.81	
Asv (mm2)	65.96	53.45	
As (mm2)	24.43	24.43	
As' (mm2)	0.00	0.00	
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	21.1		20.8
v (N/mm2)	0.26		0.25
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		1.4	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 5.81 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

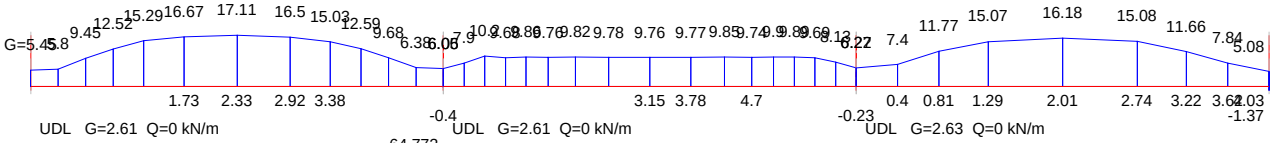
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 2 Storey: 2

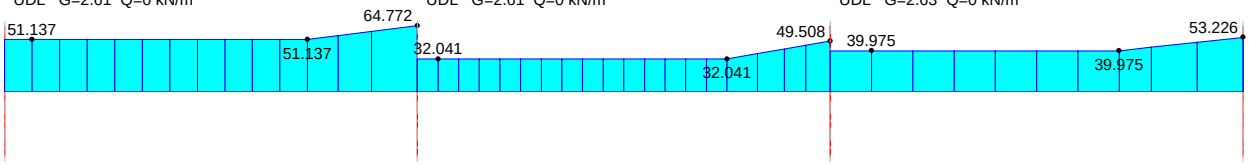
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B3 L=4650mm	2B2 L=6300mm	2B1 L=4025mm
Bw/H (mm)	250 / 500	250 / 500	250 / 500
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

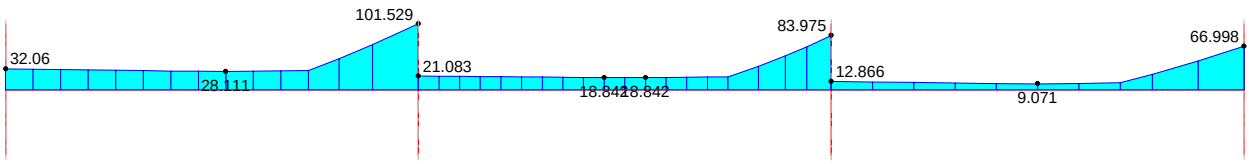
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	32.1	29.9	101.5	21.1	19.7	84.0	12.9	14.9	67.0
d (mm)	457.0	457.0	457.0	457.0	457.0	457.0	457.0	457.0	457.0
K/K'	0.12	0.11	0.38	0.08	0.07	0.31	0.05	0.06	0.25
x (mm)	57.13	57.13	84.69	57.13	57.13	69.02	57.13	57.13	57.13
Asm (mm2)	184.62	172.44	599.88	121.41	113.41	488.92	74.09	86.09	385.80
Asv (mm2)	176.32	159.82	182.93	176.11	100.14	168.74	152.01	128.40	145.08
As (mm2)	184.62	332.26	599.88	121.41	213.55	488.92	86.09	214.49	385.80
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	165.64	165.64	165.64	165.64	165.64	165.64	165.64	165.64	165.64

Bending (Bottom Edge) ...

M (kN.m)									
d (mm)									
K/K'									
x (mm)									
Asm (mm2)									
Asv (mm2)									
As (mm2)									
As' (mm2)									
As-min	159.47	165.64	159.47	159.47	165.64	159.47	159.47	165.64	159.47

Shear and Bending Design ...

Vd (kN)	56.4		58.5	56.4		54.0	48.6		46.4
v (N/mm2)	0.49		0.51	0.49		0.47	0.43		0.41
v-Rdc (N/mm2)	0.46	0.00	0.46	0.46	0.00	0.46	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	207.4		207.4	207.4		207.4	207.4		207.4
Vnom (kN)		92.9			121.7			92.9	
Td (kN.m)		9.8			0.7			7.5	
T-min (kN.m)		4.9			4.9			4.9	
t-RdC (N/mm2)		1.17						1.17	
Str. Ratio		1.910						1.525	
As-T (mm2)		516.20						393.08	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	10.18 < 38.37 OK	13.79 < 44.27 OK	8.81 < 38.37 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	X 603.19	603.19	603.19	603.19	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

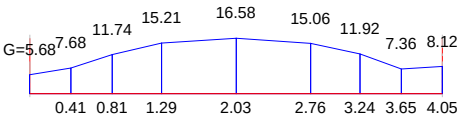
Top.Sup.	3T16		3T16	3T16		3T16	3T16		3T16
Top.Sup.									
Bot.Bars	3T16			3T16			3T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

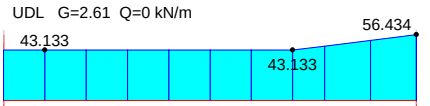
Axis: B Storey: 2

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B4	L=4050mm
Bw/H (mm)	250 / 500	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

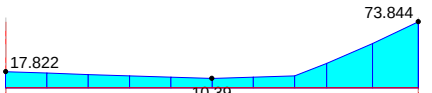
Loads





Shear Force (Envelope)

Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	17.8	18.0	73.8
d (mm)	457.0	457.0	457.0
K/K'	0.07	0.07	0.27
x (mm)	57.13	57.13	60.20
Asm (mm2)	102.63	103.39	426.43
Asv (mm2)	165.59	138.30	148.60
As (mm2)	103.39	241.70	426.43
As' (mm2)	0.00	0.00	0.00
As-min	165.64	165.64	165.64

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	159.47	165.64	159.47

Shear and Bending Design ...

Vd (kN)	53.0		47.5
v (N/mm2)	0.46		0.42
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	207.4		207.4
Vnom (kN)		92.9	
Td (kN.m)		9.7	
T-min (kN.m)		4.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.746	
As-T (mm2)		507.76	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	8.86 < 29.52 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19

Steel Bars ...

Top.Sup.	3T16		3T16
Top.Sup.			
Bot.Bars		3T16	
Bot.Bars			
Bot.Sup.	3T16		3T16
Bot.Sup.			

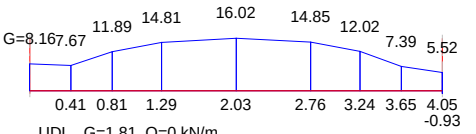
Side Bar

Axis: B Storey: 2 (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

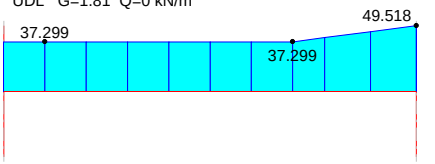
	2B6	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

Loads

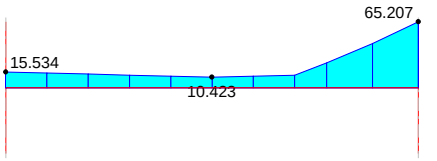


UDL G=1.81 Q=0 kN/m

Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	15.5	16.5	65.2
d (mm)	407.0	409.0	407.0
K/K'	0.09	0.10	0.38
x (mm)	50.88	51.13	76.42
Asm (mm2)	100.44	105.85	433.06
Asv (mm2)	146.17	119.79	145.22
As (mm2)	105.85	225.64	433.06
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	46.8		46.5
v (N/mm2)	0.57		0.57
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		4.9	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		2.126	
As-T (mm2)		312.71	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.9 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

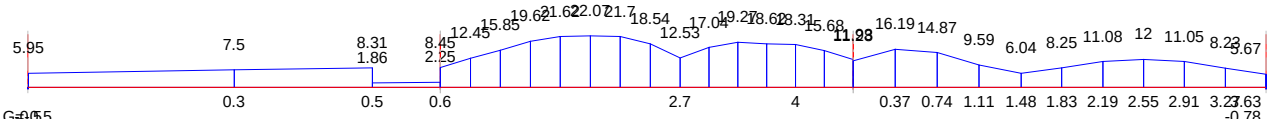
Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 3 Storey: 2

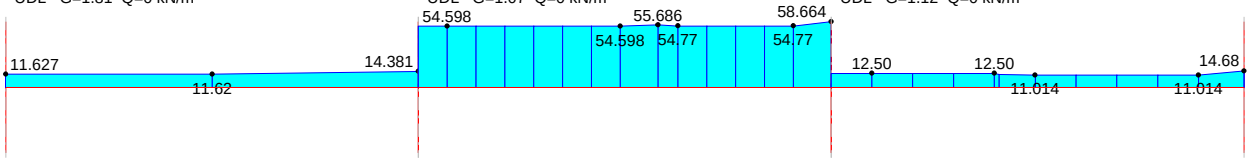
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B11 L=600mm	2B10 L=4650mm	2B9 L=3625mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

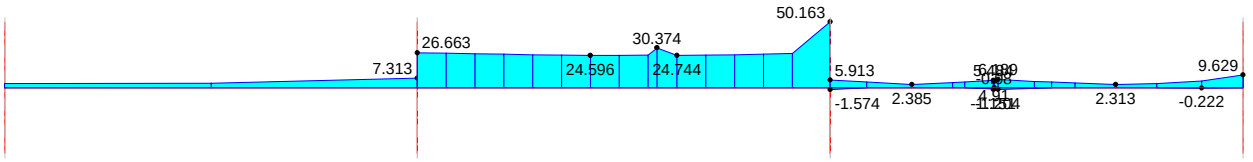
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	3.2	3.7	7.3	26.7	30.4	50.2	5.9	6.2	9.6
d (mm)	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0	407.0
K/K'	0.02	0.02	0.04	0.16	0.18	0.29	0.03	0.04	0.06
x (mm)	50.88	50.88	50.88	50.88	50.88	57.64	50.88	50.88	50.88
Asm (mm2)	20.98	24.17	47.29	172.40	196.39	326.63	38.23	40.02	62.26
Asv (mm2)	32.60	36.53	30.56	242.28	174.03	244.48	75.42	35.70	28.26
As (mm2)	24.17	47.29	47.29	196.39	326.63	326.63	40.02	62.26	62.26
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	118.01	118.01	118.01	118.01	118.01	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)							1.6	1.2	0.0
d (mm)							390.0	407.0	390.0
K/K'							0.01	0.01	0.00
x (mm)							48.75	50.88	48.75
Asm (mm2)							10.62	7.78	0.28
Asv (mm2)							75.42	35.70	23.84
As (mm2)							10.62	10.62	7.78
As' (mm2)							0.00	0.00	0.00
As-min	0.00	118.01	113.08	113.08	118.01	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	14.4		15.3	77.5		78.2	24.1		16.1
v (N/mm2)	0.18		0.19	0.95		0.96	0.30		0.20
v-Rdc (N/mm2)	0.53	0.00	0.53	0.53	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	191.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			66.5	
Td (kN.m)		2.8			1.4			1.7	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	1.47 < 10.47 OK	11.43 < 39.28 OK	8.91 < 39.28 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

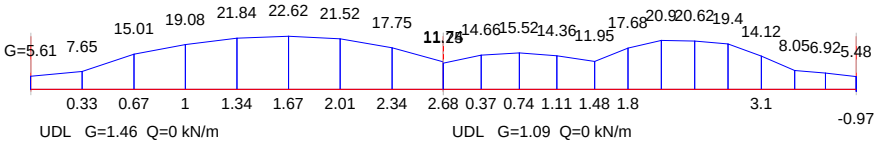
Top.Sup.			3T16	3T16		3T16	3T16		3T16
Top.Sup.									
Bot.Bars	3T16			3T16			3T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 3 Storey: 2 ...

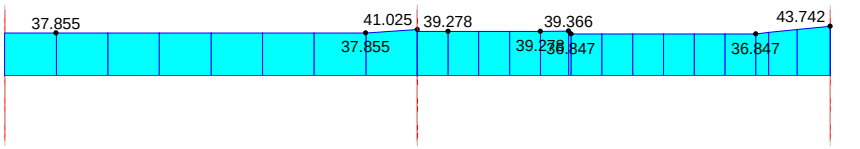
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B8 L=2675mm	2B7 L=4025mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

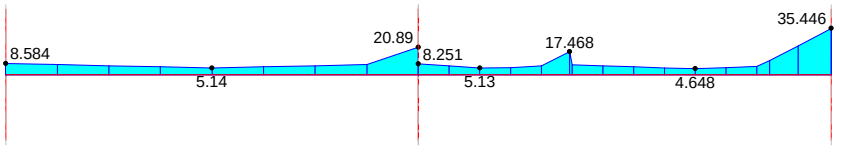
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	8.6	6.6	20.9	8.3	17.5	35.4
d (mm)	407.0	407.0	407.0	407.0	407.0	407.0
K/K'	0.05	0.04	0.12	0.05	0.10	0.21
x (mm)	50.88	50.88	50.88	50.88	50.88	50.88
Asm (mm2)	55.50	42.59	135.07	53.35	112.94	229.19
Asv (mm2)	119.58	118.30	153.10	89.21	123.03	120.96
As (mm2)	55.50	135.07	135.07	112.94	229.19	229.19
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.01	118.01	118.01	118.01	118.01	118.01

Bending (Bottom Edge) ...

M (kN.m)						
d (mm)						
K/K'						
x (mm)						
Asm (mm2)						
Asv (mm2)						
As (mm2)						
As' (mm2)						
As-min	113.08	118.01	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	38.3		49.0	28.5		38.7
v (N/mm2)	0.47		0.60	0.35		0.48
v-Rdc (N/mm2)	0.53	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5	
Td (kN.m)		3.4			1.2	
T-min (kN.m)		2.9			2.9	
t-RdC (N/mm2)		1.17				
Str. Ratio		1.338				
As-T (mm2)		218.41				
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.57 < 39.28 OK	9.89 < 34.04 OK

Supplied Steel Areas (mm2)

Top Edge	603.19	603.19	603.19	603.19	603.19	603.19
Bot Edge	603.19	603.19	603.19	603.19	603.19	603.19

Steel Bars ...

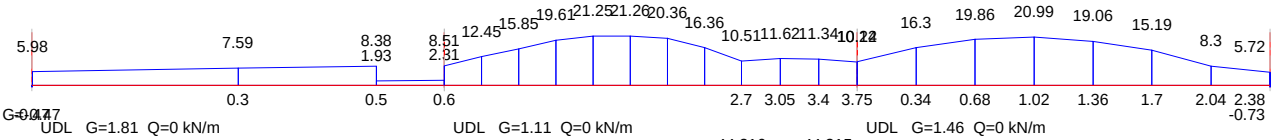
Top.Sup.	3T16		3T16	3T16		3T16
Top.Sup.						
Bot.Bars		3T16			3T16	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: 5 Storey: 2

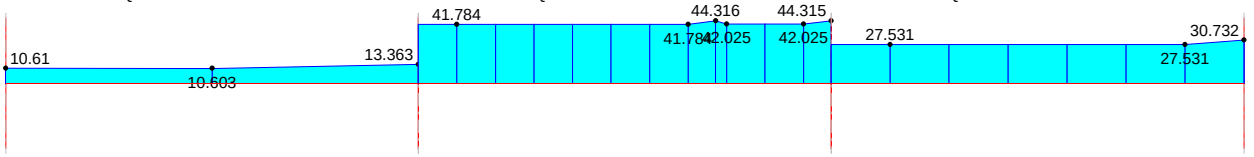
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B18 L=600mm	2B17 L=3750mm	2B16 L=2375mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

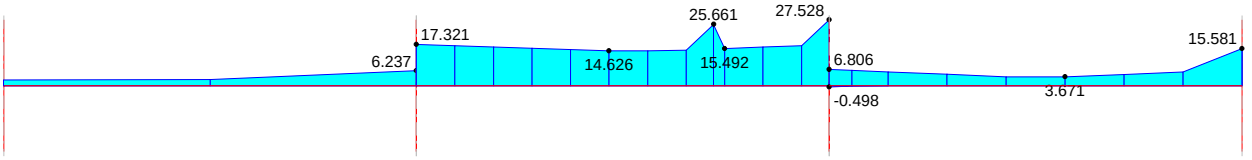
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	2.5	2.9	6.2	17.3	25.7	27.5	6.8	4.7	15.6
d (mm)	390.0	390.0	399.0	399.0	409.0	407.0	407.0	409.0	409.0
K/K'	0.02	0.02	0.04	0.11	0.15	0.16	0.04	0.03	0.09
x (mm)	48.75	48.75	49.88	49.88	51.13	50.88	50.88	51.13	51.13
Asm (mm2)	16.62	19.85	41.14	114.24	165.11	177.99	44.01	30.46	100.25
Asv (mm2)	33.16	33.85	54.00	190.87	138.50	170.87	143.55	86.04	81.35
As (mm2)	19.85	41.14	41.14	165.11	177.99	177.99	44.01	100.25	100.25
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	113.08	115.69	115.69	118.59	118.01	118.01	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)							0.5		
d (mm)							390.0		
K/K'							0.00		
x (mm)							48.75		
Asm (mm2)							3.36		
Asv (mm2)							143.55		
As (mm2)							3.36		
As' (mm2)							0.00		
As-min	0.00	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	12.7		17.3	61.1		54.7	45.9		26.0
v (N/mm2)	0.16		0.22	0.77		0.67	0.56		0.32
v-Rdc (N/mm2)	0.75	0.00	0.73	0.73	0.00	0.46	0.46	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	192.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			108.8	
Td (kN.m)		2.5			0.8			0.0	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	1.5 < 8.39 OK	9.17 < 88.02 OK	5.81 < 88.5 OK

Supplied Steel Areas (mm2)

Top Edge	1963.50	1963.50	1608.49	1608.49	226.19	402.12	402.12	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

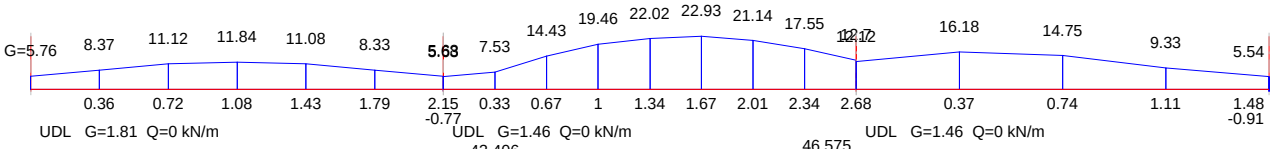
Top.Sup.			2T32	2T32		2T16	2T16		2T12
Top.Sup.									
Bot.Bars	2T12			2T12			2T12		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 5 Storey: 2 ...

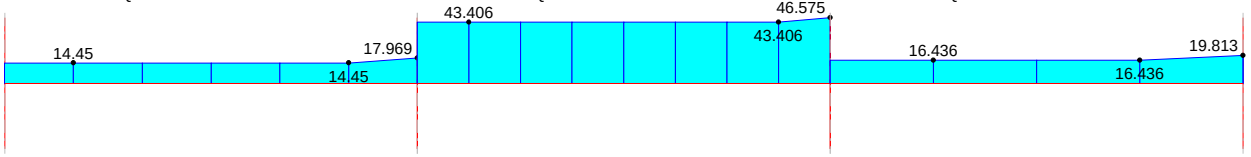
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B15 L=2150mm	2B14 L=2675mm	2B13 L=1475mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

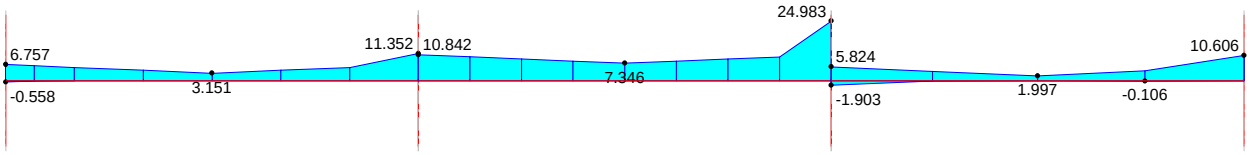
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	6.8	4.6	11.4	10.8	8.8	25.0	5.8	3.4	10.6
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.04	0.03	0.07	0.06	0.05	0.14	0.03	0.02	0.06
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	43.48	29.62	73.04	69.76	56.83	160.74	37.47	21.87	68.24
Asv (mm2)	72.82	32.80	55.13	109.48	135.65	163.44	73.13	27.26	57.13
As (mm2)	43.48	62.42	73.04	69.76	160.74	160.74	37.47	49.13	68.24
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)	0.6						1.9		0.1
d (mm)	390.0						390.0		390.0
K/K'	0.00						0.01		0.00
x (mm)	48.75						48.75		48.75
Asm (mm2)	3.76						12.84		0.70
Asv (mm2)	72.82						73.13		57.13
As (mm2)	3.76						12.84		0.70
As' (mm2)	0.00						0.00		0.00
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	23.3		17.6	35.0		52.3	23.4		18.3
v (N/mm2)	0.28		0.22	0.43		0.64	0.29		0.22
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			108.8	
Td (kN.m)		4.7			1.6			0.3	
T-min (kN.m)		2.9			2.9			2.9	
t-RdC (N/mm2)		1.17							
Str. Ratio		1.137							
As-T (mm2)		302.11							
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.26 < 88.5 OK	6.54 < 88.5 OK	3.61 < 88.5 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

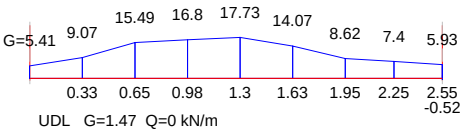
Top.Sup.	2T12		2T12	2T12		2T12	2T12		2T12
Top.Sup.									
Bot.Bars	2T12			2T12			2T12		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: 5 Storey: 2 ...

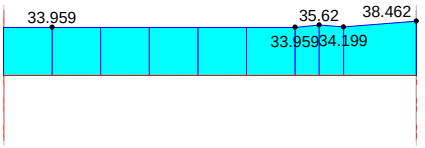
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B12	L=2550mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

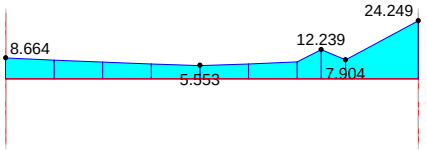
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	8.7	7.5	24.2
d (mm)	409.0	409.0	409.0
K/K'	0.05	0.04	0.14
x (mm)	51.13	51.13	51.13
Asm (mm2)	55.75	48.06	156.02
Asv (mm2)	88.21	106.56	120.80
As (mm2)	55.75	154.62	156.02
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	28.2		38.7
v (N/mm2)	0.35		0.47
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		1.0	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 6.23 < 76.7 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

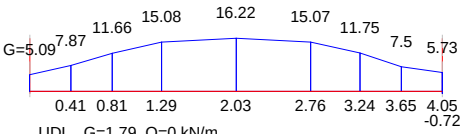
Axis: M Storey: 2

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

2B19 L=4050mm

Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

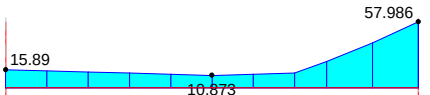
Loads



UDL G=1.79 Q=0 kN/m

Shear Force (Envelope)

Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	15.9	16.2	58.0
d (mm)	407.0	409.0	407.0
K/K'	0.09	0.09	0.34
x (mm)	50.88	51.13	67.31
Asm (mm2)	102.74	104.21	381.41
Asv (mm2)	145.89	100.26	143.46
As (mm2)	104.21	204.48	381.41
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	46.7		45.9
v (N/mm2)	0.57		0.56
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		3.0	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.702	
As-T (mm2)		193.20	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.9 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	X 402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

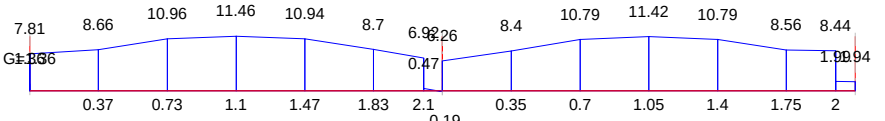
Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: L Storey: 2

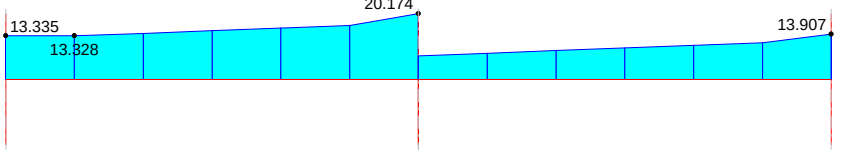
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B20 L=2200mm	2B21 L=2100mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

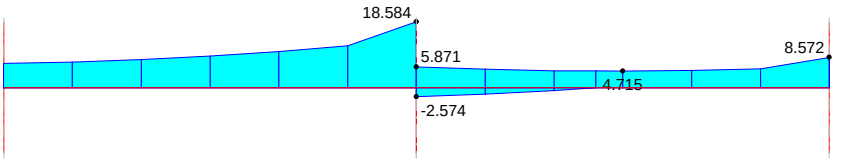
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	7.6	10.5	18.6	5.9	5.1	8.6
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.04	0.06	0.11	0.03	0.03	0.05
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	49.20	67.68	119.57	37.77	32.76	55.15
Asv (mm2)	36.07	29.39	83.63	85.31	33.66	31.84
As (mm2)	67.68	97.07	119.57	37.77	55.15	55.15
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				2.6	1.0	
d (mm)				390.0	409.0	
K/K'				0.02	0.01	
x (mm)				48.75	51.13	
Asm (mm2)				17.37	6.17	
Asv (mm2)				85.31	33.66	
As (mm2)				17.37	17.37	
As' (mm2)				0.00	0.00	
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	12.4		26.8	27.3		10.2
v (N/mm2)	0.15		0.33	0.33		0.12
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			108.8	
Td (kN.m)		1.0			0.6	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d		5.38 < 76.7 OK			5.13 < 76.7 OK	

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

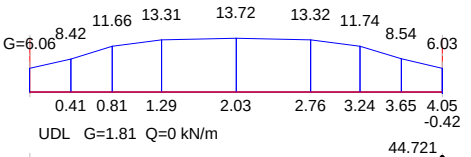
Top.Sup.	2T12		2T12	2T12		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: M Storey: 2 (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

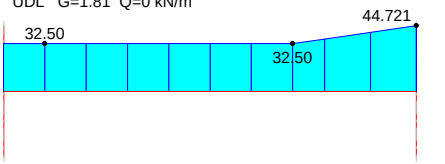
	2B22	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

Loads

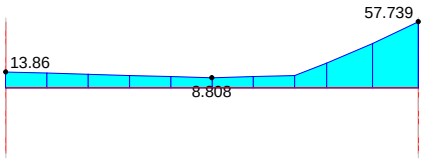


UDL G=1.81 Q=0 kN/m

Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	13.9	14.3	57.7
d (mm)	409.0	409.0	409.0
K/K'	0.08	0.08	0.33
x (mm)	51.13	51.13	66.62
Asm (mm2)	89.18	92.14	377.53
Asv (mm2)	135.62	104.79	134.41
As (mm2)	92.14	196.93	377.53
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	43.4		43.0
v (N/mm2)	0.53		0.53
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		2.5	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.9 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

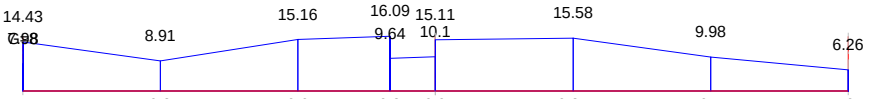
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 4 Storey: 2

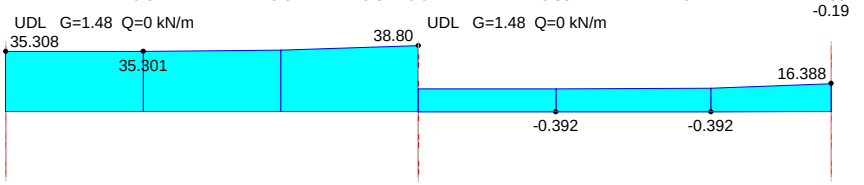
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B24 L=900mm	2B23 L=1050mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

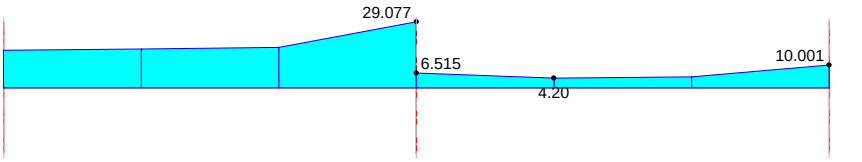
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	16.9	17.7	29.1	6.5	4.7	10.0
d (mm)	390.0	390.0	399.0	399.0	409.0	409.0
K/K'	0.11	0.11	0.18	0.04	0.03	0.06
x (mm)	48.75	48.75	49.88	49.88	51.13	51.13
Asm (mm2)	114.31	119.48	191.78	42.97	30.40	64.35
Asv (mm2)	159.81	112.40	206.88	67.13	34.83	34.95
As (mm2)	119.48	191.78	191.78	42.97	64.35	64.35
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	113.08	115.69	115.69	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)						
d (mm)						
K/K'						
x (mm)						
Asm (mm2)						
Asv (mm2)						
As (mm2)						
As' (mm2)						
As-min	0.00	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	51.1		66.2	21.5		13.2
v (N/mm2)	0.66		0.83	0.27		0.16
v-Rdc (N/mm2)	0.75	0.00	0.73	0.73	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	192.8		183.8	183.8		183.8
Vnom (kN)		108.8			108.8	
Td (kN.m)		0.1			0.0	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d		2.26 < 8.39 OK			---	

Supplied Steel Areas (mm2)

Top Edge	1963.50	1963.50	1608.49	1608.49	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

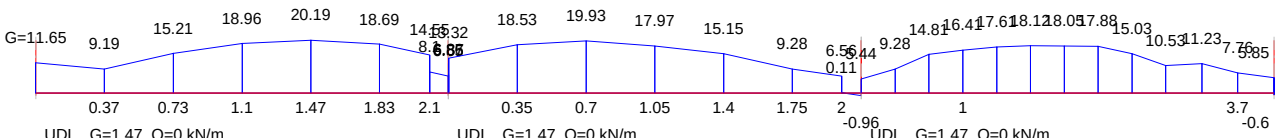
Top.Sup.			2T32	2T32		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: J Storey: 2

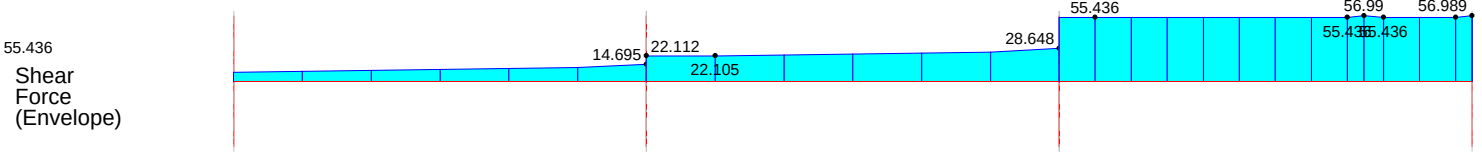
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B25 L=2200mm	2B26 L=2100mm	2B28 L=4050mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

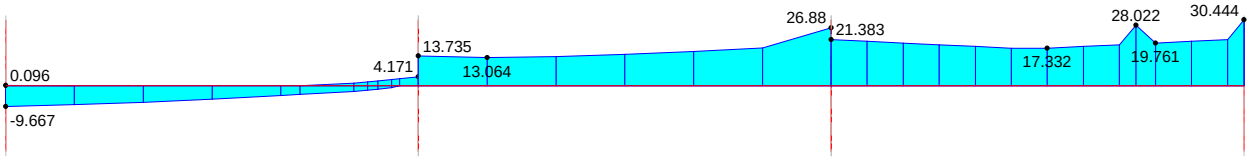
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	2.0	0.4	4.2	13.7	16.1	26.9	21.4	25.1	30.4
d (mm)	409.0	409.0	409.0	409.0	409.0	407.0	407.0	409.0	407.0
K/K'	0.01	0.00	0.02	0.08	0.09	0.16	0.12	0.15	0.18
x (mm)	51.13	51.13	51.13	51.13	51.13	50.88	50.88	51.13	50.88
Asm (mm2)	12.65	2.45	26.83	88.37	103.51	173.80	138.26	161.46	196.84
Asv (mm2)	128.75	36.03	79.82	70.52	76.92	118.19	176.66	176.54	176.91
As (mm2)	12.65	26.83	26.83	103.51	173.80	173.80	161.46	196.84	196.84
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.01	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)	9.7	7.9	2.9						
d (mm)	390.0	409.0	390.0						
K/K'	0.06	0.05	0.02						
x (mm)	48.75	51.13	48.75						
Asm (mm2)	65.23	50.58	19.65						
Asv (mm2)	128.75	36.03	79.82						
As (mm2)	65.23	65.23	50.58						
As' (mm2)	0.00	0.00	0.00						
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	41.2		25.5	22.6		37.8	56.5		56.6
v (N/mm2)	0.50		0.31	0.28		0.46	0.69		0.70
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.46	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		108.8			66.5			66.5	
Td (kN.m)		0.6			1.6			1.5	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.38 < 76.7 OK	5.13 < 88.5 OK	9.9 < 71.92 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	402.12	402.12	226.19	402.12
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T12		2T12	2T12		2T16	2T16		2T16
Top.Sup.									
Bot.Bars	2T12			2T12			2T12		
Bot.Bars									
Bot.Sup.									
Side Bar									

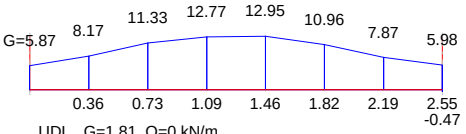
Axis: 6 Storey: 2

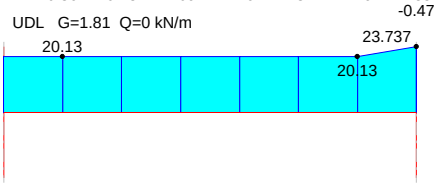
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

2B27 L=2550mm

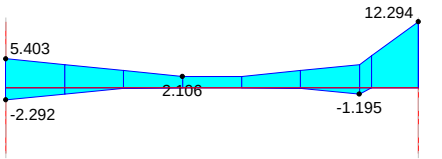
Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

Loads





Shear Force (Envelope)



Moment (Envelope)

Bending (Top Edge) ...

M (kN.m)	5.4	3.2	12.3
d (mm)	409.0	409.0	409.0
K/K'	0.03	0.02	0.07
x (mm)	51.13	51.13	51.13
Asm (mm2)	34.77	20.46	79.10
Asv (mm2)	78.02	62.91	77.79
As (mm2)	34.77	79.10	79.10
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)	2.3	0.1	0.7
d (mm)	390.0	409.0	390.0
K/K'	0.01	0.00	0.00
x (mm)	48.75	51.13	48.75
Asm (mm2)	15.46	0.44	4.84
Asv (mm2)	78.02	62.91	77.79
As (mm2)	15.46	15.46	4.84
As' (mm2)	0.00	0.00	0.00
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	25.0		24.9
v (N/mm2)	0.31		0.30
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		3.4	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.147	
As-T (mm2)		216.01	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.23 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

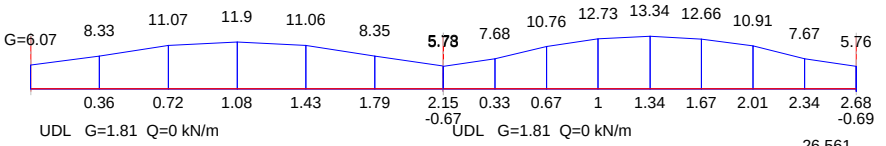
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 7 Storey: 2

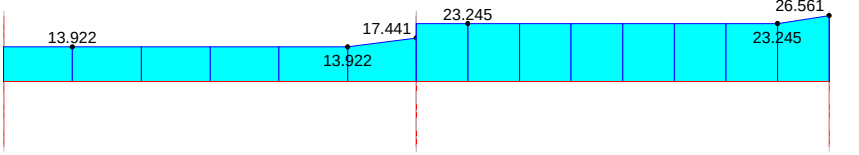
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B30 L=2150mm	2B29 L=2675mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

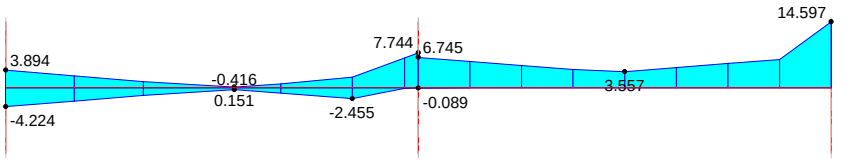
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	3.9	1.6	7.7	6.7	5.1	14.6
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.02	0.01	0.04	0.04	0.03	0.08
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	25.06	10.52	49.82	43.40	32.77	93.92
Asv (mm2)	79.73	33.78	29.25	79.98	72.65	84.03
As (mm2)	25.06	44.30	49.82	43.40	93.92	93.92
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)	4.2	2.0	2.1	0.1		
d (mm)	390.0	409.0	390.0	390.0		
K/K'	0.03	0.01	0.01	0.00		
x (mm)	48.75	51.13	48.75	48.75		
Asm (mm2)	28.50	12.65	13.85	0.60		
Asv (mm2)	79.73	33.78	24.92	79.98		
As (mm2)	28.50	28.50	13.85	0.60		
As' (mm2)	0.00	0.00	0.00	0.00		
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	25.5		15.9	25.6		26.9
v (N/mm2)	0.31		0.19	0.31		0.33
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5	
Td (kN.m)		3.9			5.6	
T-min (kN.m)		2.9			2.9	
t-RdC (N/mm2)		1.17			1.17	
Str. Ratio		1.018			1.513	
As-T (mm2)		248.98			360.01	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.26 < 76.7 OK			6.54 < 76.7 OK		

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

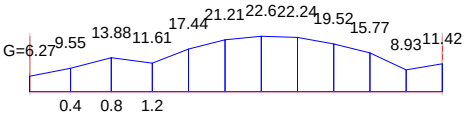
Top.Sup.	2T12		2T12	2T12		2T12
Top.Sup.						
Bot.Bars	2T12			2T12		
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: I Storey: 2

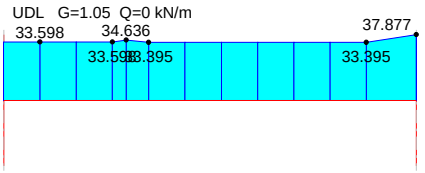
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B31	L=4050mm
Bw/H (mm)	200 / 450	
Flange Bf/Hf	---	
	C25/30 / Grade 460 (Type 2)	

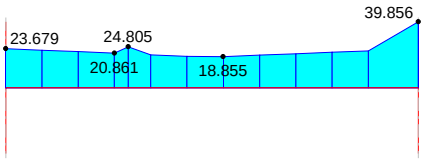
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	23.7	24.8	39.9
d (mm)	407.0	409.0	407.0
K/K'	0.14	0.14	0.23
x (mm)	50.88	51.13	50.88
Asm (mm2)	153.10	159.60	257.70
Asv (mm2)	209.68	108.24	210.83
As (mm2)	159.60	257.70	257.70
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	67.1		67.5
v (N/mm2)	0.82		0.83
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		0.8	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.9 < 43.97 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

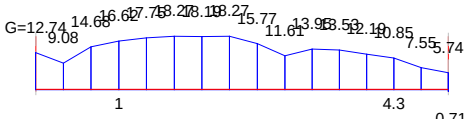
Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: I Storey: 2 (/ 2)

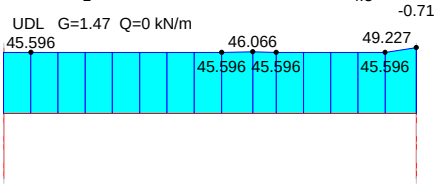
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B32		L=4950mm
Bw/H (mm)	200 / 450		
Flange Bf/Hf	---		
	C25/30 / Grade 460 (Type 2)		

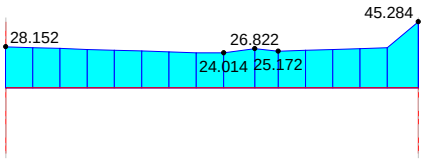
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	28.2	26.8	45.3
d (mm)	405.0	409.0	405.0
K/K'	0.17	0.16	0.27
x (mm)	50.63	51.13	52.00
Asm (mm2)	182.92	172.57	294.67
Asv (mm2)	227.70	143.97	203.64
As (mm2)	182.92	294.67	294.67
As' (mm2)	0.00	0.00	0.00
As-min	117.43	118.59	117.43

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	72.9		65.2
v (N/mm2)	0.90		0.80
v-Rdc (N/mm2)	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		3.7	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.717	
As-T (mm2)		237.88	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	12.16 < 30.35 OK

Supplied Steel Areas (mm2)

Top Edge	628.32	X 226.19	628.32
Bot Edge	402.12	402.12	402.12

Steel Bars ...

Top.Sup.	2T20		2T20
Top.Sup.			
Bot.Bars		2T16	
Bot.Bars			
Bot.Sup.			
Side Bar			

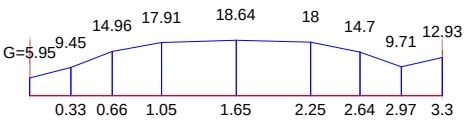
Axis: 4 Storey: 2 (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

2B33 L=3300mm

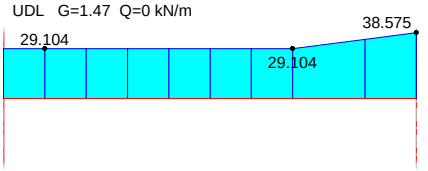
Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

Loads

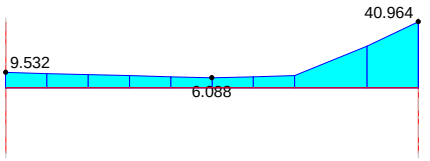


UDL G=1.47 Q=0 kN/m

Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	9.5	9.5	41.0
d (mm)	407.0	409.0	409.0
K/K'	0.06	0.06	0.24
x (mm)	50.88	51.13	51.13
Asm (mm2)	61.63	61.35	263.57
Asv (mm2)	155.29	92.90	134.74
As (mm2)	61.63	154.24	263.57
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	49.7		43.1
v (N/mm2)	0.61		0.53
v-Rdc (N/mm2)	0.46	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		108.8	
Td (kN.m)		0.4	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	8.07 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T16		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

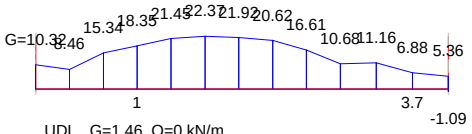
Axis: D Storey: 2

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

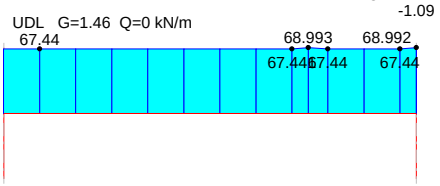
2B34 L=4050mm

Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

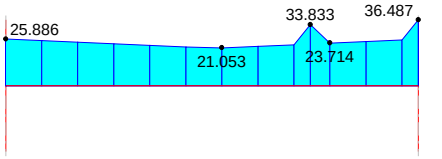
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	25.9	30.3	36.5
d (mm)	407.0	409.0	407.0
K/K'	0.15	0.18	0.21
x (mm)	50.88	51.13	50.88
Asm (mm2)	167.38	194.84	235.92
Asv (mm2)	206.00	214.05	220.61
As (mm2)	194.84	235.92	235.92
As' (mm2)	0.00	0.00	0.00
As-min	118.01	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	65.9		70.6
v (N/mm2)	0.81		0.87
v-Rdc (N/mm2)	0.46	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		108.8	
Td (kN.m)		0.1	
T-min (kN.m)		2.9	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d 9.9 < 43.52 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	X 226.19	402.12
Bot Edge	226.19	226.19	226.19

Steel Bars ...

Top.Sup.	2T16		2T16
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

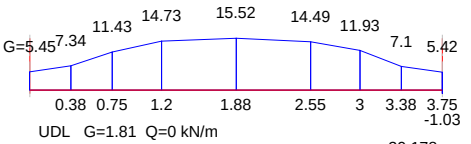
Axis: 6 Storey: 2 (/ 2)

Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

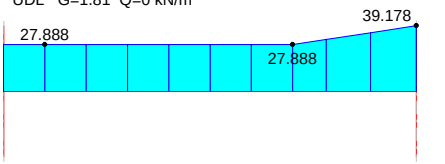
2B35 L=3750mm

Bw/H (mm)	200 / 450
Flange Bf/Hf	---
	C25/30 / Grade 460 (Type 2)

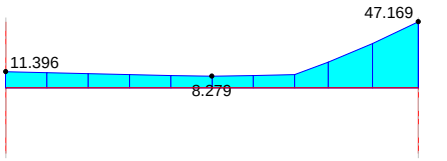
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	11.4	12.0	47.2
d (mm)	409.0	409.0	409.0
K/K'	0.07	0.07	0.27
x (mm)	51.13	51.13	53.70
Asm (mm2)	73.33	77.33	304.30
Asv (mm2)	133.78	89.90	121.89
As (mm2)	77.33	167.23	304.30
As' (mm2)	0.00	0.00	0.00
As-min	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)			
d (mm)			
K/K'			
x (mm)			
Asm (mm2)			
Asv (mm2)			
As (mm2)			
As' (mm2)			
As-min	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	42.8		39.0
v (N/mm2)	0.52		0.48
v-Rdc (N/mm2)	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05
V-Rd (kN)	183.8		183.8
Vnom (kN)		66.5	
Td (kN.m)		4.3	
T-min (kN.m)		2.9	
t-RdC (N/mm2)		1.17	
Str. Ratio		1.743	
As-T (mm2)		278.68	
b-sup (mm)	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	9.17 < 59.0 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	X 226.19	X 226.19
Bot Edge	226.19	226.19	226.19

Steel Bars ...

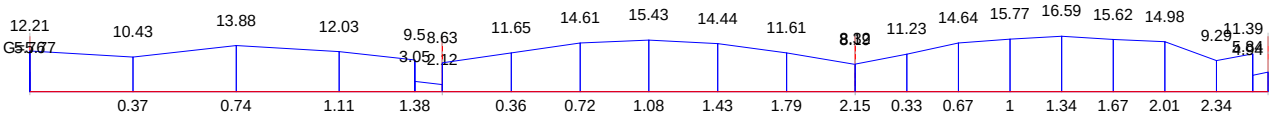
Top.Sup.	2T12		2T12
Top.Sup.			
Bot.Bars		2T12	
Bot.Bars			
Bot.Sup.			
Side Bar			

Axis: 2a Storey: 2

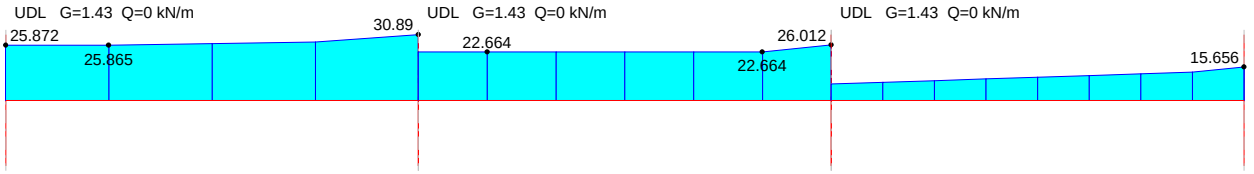
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B41 L=1475mm	2B40 L=2150mm	2B39 L=2675mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

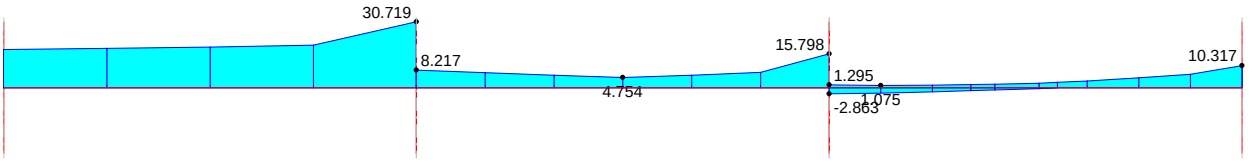
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	18.4	19.6	30.7	8.2	6.1	15.8	1.3	4.5	10.3
d (mm)	407.0	407.0	407.0	407.0	409.0	407.0	407.0	409.0	409.0
K/K'	0.11	0.11	0.18	0.05	0.04	0.09	0.01	0.03	0.06
x (mm)	50.88	50.88	50.88	50.88	51.13	50.88	50.88	51.13	51.13
Asm (mm2)	118.80	126.94	198.62	53.13	39.49	102.15	8.37	28.64	66.38
Asv (mm2)	65.34	84.97	130.48	93.24	70.83	76.71	145.97	36.08	58.09
As (mm2)	126.94	198.62	198.62	53.13	102.15	102.15	28.64	64.72	66.38
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.01	118.01	118.01	118.01	118.59	118.01	118.01	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)							2.9	1.7	
d (mm)							390.0	409.0	
K/K'							0.02	0.01	
x (mm)							48.75	51.13	
Asm (mm2)							19.32	10.97	
Asv (mm2)							145.97	36.08	
As (mm2)							19.32	19.32	
As' (mm2)							0.00	0.00	
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	20.9		41.8	29.8		24.5	46.7		18.6
v (N/mm2)	0.26		0.51	0.37		0.30	0.57		0.23
v-Rdc (N/mm2)	0.46	0.00	0.46	0.46	0.00	0.46	0.46	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5			66.5	
Td (kN.m)		1.0			2.1			0.9	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	3.61 < 76.7 OK	5.26 < 88.5 OK	6.54 < 76.7 OK

Supplied Steel Areas (mm2)

Top Edge	402.12	402.12	402.12	402.12	226.19	402.12	402.12	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

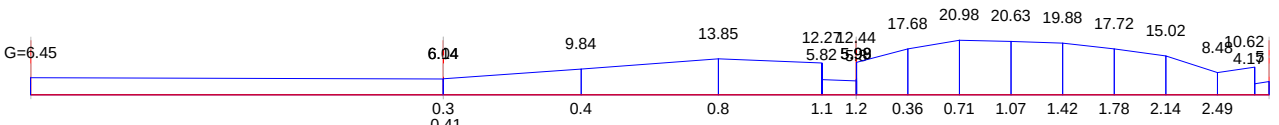
Top.Sup.			2T16	2T16		2T16	2T16		2T12
Top.Sup.									
Bot.Bars	2T12			2T12			2T12		
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: E Storey: 2

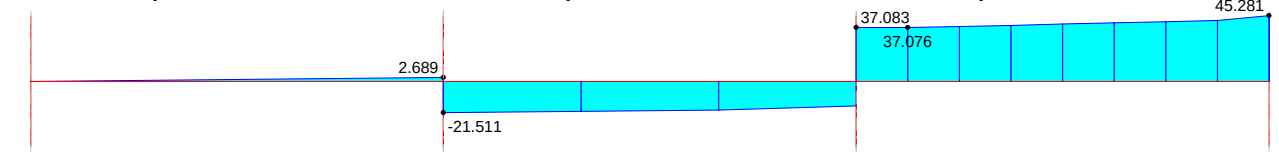
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B42 L=300mm	2B43 L=1203.6mm	2B44 L=2846.4mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

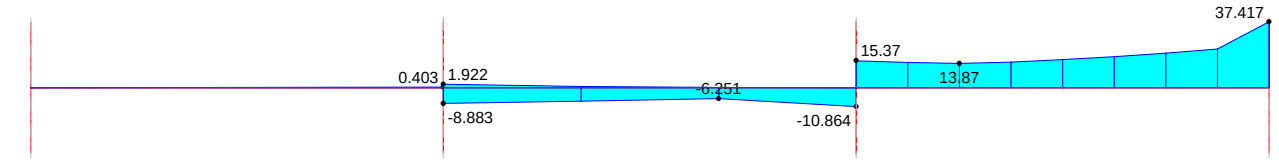
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	0.0	0.1	0.4	1.9	0.7	1.9	15.4	18.9	37.4
d (mm)	390.0	390.0	399.0	399.0	407.0	409.0	409.0	409.0	407.0
K/K'	0.00	0.00	0.00	0.01	0.00	0.01	0.09	0.11	0.22
x (mm)	48.75	48.75	49.88	49.88	50.88	51.13	51.13	51.13	50.88
Asm (mm2)	0.23	0.68	2.66	12.68	4.79	12.10	98.90	121.83	241.93
Asv (mm2)	0.82	2.10	1.90	140.15	64.55	96.90	106.63	127.46	175.03
As (mm2)	0.68	2.66	2.66	12.68	12.68	12.10	121.83	241.93	241.93
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	0.00	113.08	115.69	115.69	118.01	118.59	118.59	118.59	118.01

Bending (Bottom Edge) ...

M (kN.m)				8.9	7.5	8.2			
d (mm)				390.0	409.0	390.0			
K/K'				0.06	0.04	0.05			
x (mm)				48.75	51.13	48.75			
Asm (mm2)				59.94	48.40	55.01			
Asv (mm2)				140.15	64.55	96.90			
As (mm2)				59.94	59.94	55.01			
As' (mm2)				0.00	0.00	0.00			
As-min	0.00	118.59	113.08	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	0.3		0.6	44.8		31.0	34.1		56.0
v (N/mm2)	0.00		0.01	0.56		0.38	0.42		0.69
v-Rdc (N/mm2)	0.75	0.00	0.73	0.73	0.00	0.38	0.38	0.00	0.46
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	192.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		108.8			66.5			66.5	
Td (kN.m)		0.0			2.2			4.0	
T-min (kN.m)		2.9			2.9			2.9	
t-RdC (N/mm2)								1.17	
Str. Ratio								1.888	
As-T (mm2)								256.73	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links			**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	0.75 < 8.39 OK	2.94 < 88.5 OK	6.96 < 72.82 OK

Supplied Steel Areas (mm2)

Top Edge	1963.50	1963.50	1608.49	1608.49	402.12	226.19	226.19	X 226.19	402.12
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

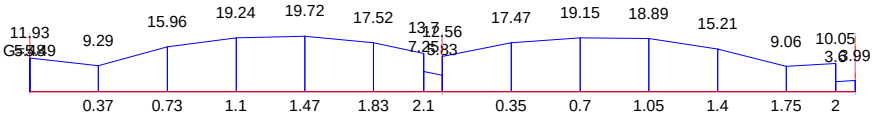
Top.Sup.			2T32	2T32		2T12	2T12		2T16
Top.Sup.									
Bot.Bars		2T12			2T12			2T12	
Bot.Bars									
Bot.Sup.									
Side Bar									

Axis: C Storey: 2

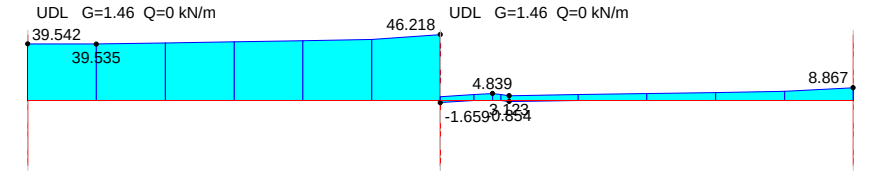
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B45 L=2200mm	2B46 L=2100mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

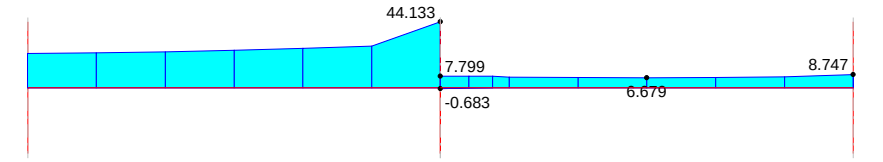
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	23.8	26.5	44.1	7.8	7.1	8.7
d (mm)	407.0	407.0	407.0	407.0	409.0	409.0
K/K'	0.14	0.15	0.26	0.05	0.04	0.05
x (mm)	50.88	50.88	50.88	50.88	51.13	51.13
Asm (mm2)	153.85	171.55	285.35	50.43	45.51	56.28
Asv (mm2)	36.01	131.43	178.46	155.77	18.40	34.63
As (mm2)	171.55	285.35	285.35	50.43	56.28	56.28
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.01	118.01	118.01	118.01	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				0.7		
d (mm)				390.0		
K/K'				0.00		
x (mm)				48.75		
Asm (mm2)				4.61		
Asv (mm2)				155.77		
As (mm2)				4.61		
As' (mm2)				0.00		
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	12.3		57.1	49.8		11.1
v (N/mm2)	0.15		0.70	0.61		0.14
v-Rdc (N/mm2)	0.46	0.00	0.46	0.46	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			66.5	
Td (kN.m)		1.1			1.2	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d		5.38 < 76.7 OK			5.13 < 76.7 OK	

Supplied Steel Areas (mm2)

Top Edge	402.12	402.12	402.12	402.12	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

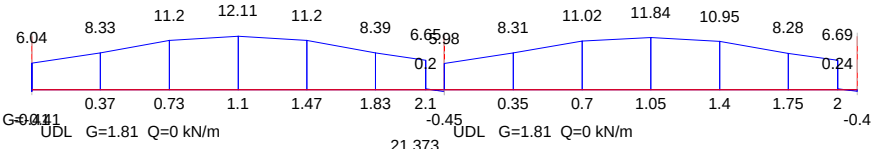
Top.Sup.			2T16	2T16		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: A Storey: 2

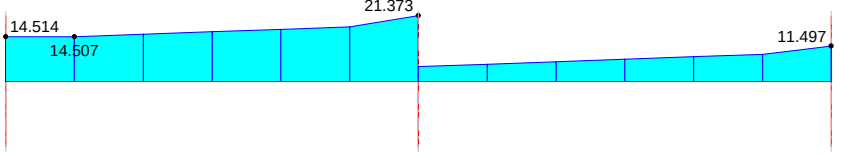
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B47 L=2200mm	2B48 L=2100mm
Bw/H (mm)	200 / 450	200 / 450
Flange Bf/Hf	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

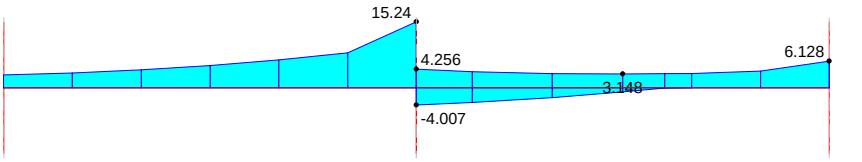
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	3.8	6.7	15.2	4.3	3.5	6.1
d (mm)	409.0	409.0	409.0	409.0	409.0	409.0
K/K'	0.02	0.04	0.09	0.02	0.02	0.04
x (mm)	51.13	51.13	51.13	51.13	51.13	51.13
Asm (mm2)	24.64	43.30	98.06	27.38	22.82	39.43
Asv (mm2)	35.18	23.30	81.00	80.01	26.06	35.30
As (mm2)	43.30	66.60	98.06	27.38	39.43	39.43
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.59	118.59	118.59	118.59

Bending (Bottom Edge) ...

M (kN.m)				4.0	2.4	
d (mm)				390.0	409.0	
K/K'				0.03	0.01	
x (mm)				48.75	51.13	
Asm (mm2)				27.04	15.48	
Asv (mm2)				80.01	26.06	
As (mm2)				27.04	27.04	
As' (mm2)				0.00	0.00	
As-min	113.08	118.59	113.08	113.08	118.59	113.08

Shear and Bending Design ...

Vd (kN)	13.1		25.9	25.6		11.3
v (N/mm2)	0.16		0.32	0.31		0.14
v-Rdc (N/mm2)	0.38	0.00	0.38	0.38	0.00	0.38
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8
Vnom (kN)		66.5			108.8	
Td (kN.m)		2.0			0.1	
T-min (kN.m)		2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	5.38 < 76.7 OK			5.13 < 76.7 OK		

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	226.19	226.19	226.19	226.19
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19

Steel Bars ...

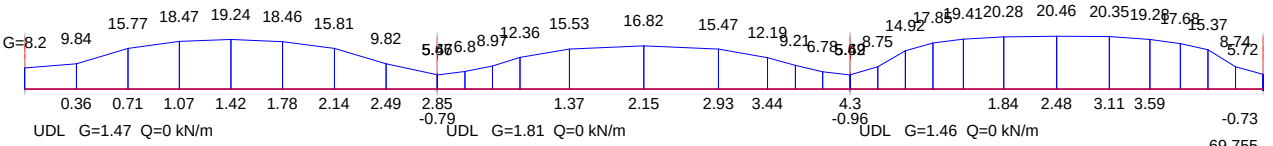
Top.Sup.	2T12		2T12	2T12		2T12
Top.Sup.						
Bot.Bars		2T12			2T12	
Bot.Bars						
Bot.Sup.						
Side Bar						

Axis: H Storey: 2

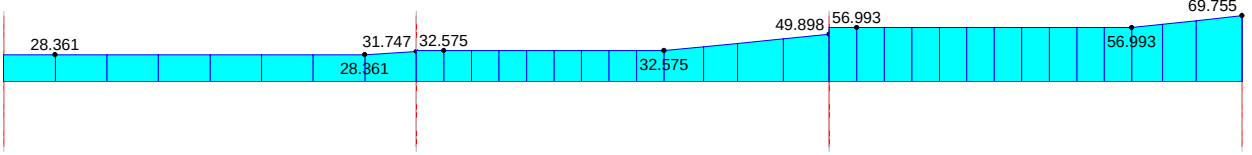
Etriyeler: C25/30 / Grade 460 (Type 2) (Links: Grade 250 (Plain))

	2B49 L=2846.4mm	2B37 L=4300mm	2B51 L=4950mm
Bw/H (mm)	200 / 450	200 / 450	200 / 450
Flange Bf/Hf	---	---	---
	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)	C25/30 / Grade 460 (Type 2)

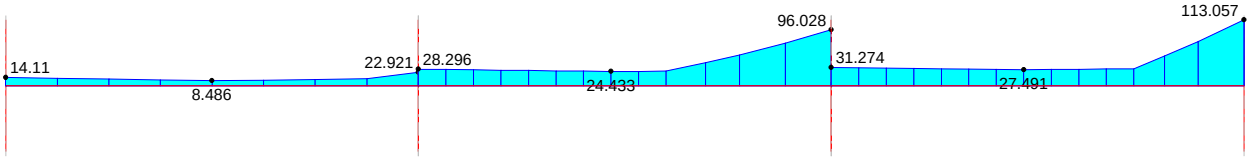
Loads



Shear Force (Envelope)



Moment (Envelope)



Bending (Top Edge) ...

M (kN.m)	14.1	10.9	22.9	28.3	44.5	96.0	31.3	29.4	113.1
d (mm)	409.0	409.0	407.0	407.0	409.0	405.0	405.0	409.0	405.0
K/K'	0.08	0.06	0.13	0.17	0.26	0.57	0.18	0.17	0.67
x (mm)	51.13	51.13	50.88	50.88	51.13	118.47	50.63	51.13	143.49
Asm (mm2)	90.79	70.15	148.20	182.95	286.31	671.31	203.21	188.98	813.12
Asv (mm2)	130.20	88.64	122.45	156.09	118.96	157.01	242.47	178.11	239.81
As (mm2)	90.79	148.20	148.20	286.31	405.27	671.31	203.21	367.09	813.12
As' (mm2)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
As-min	118.59	118.59	118.01	118.01	118.59	117.43	117.43	118.59	117.43

Bending (Bottom Edge) ...

M (kN.m)									
d (mm)									
K/K'									
x (mm)									
Asm (mm2)									
Asv (mm2)									
As (mm2)									
As' (mm2)									
As-min	113.08	118.59	113.08	113.08	118.59	113.08	113.08	118.01	113.08

Shear and Bending Design ...

Vd (kN)	41.7		39.2	49.9		50.2	77.6		76.7
v (N/mm2)	0.51		0.48	0.61		0.62	0.96		0.95
v-Rdc (N/mm2)	0.38	0.00	0.46	0.46	0.00	0.53	0.53	0.00	0.53
v-Rdmax (N/mm2)	4.05		4.05	4.05		4.05	4.05		4.05
V-Rd (kN)	183.8		183.8	183.8		183.8	183.8		183.8
Vnom (kN)		108.8			66.5			108.8	
Td (kN.m)		0.3			2.8			0.6	
T-min (kN.m)		2.9			2.9			2.9	
b-sup (mm)	0.0		0.0	0.0		0.0	0.0		0.0
Links	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**	**R10-300**

Deflection Check ...

L/d	6.96 < 76.7 OK	10.51 < 88.5 OK	12.16 < 39.41 OK

Supplied Steel Areas (mm2)

Top Edge	226.19	226.19	402.12	402.12	X 226.19	X 628.32	628.32	X 226.19	X 628.32
Bot Edge	226.19	226.19	226.19	226.19	226.19	226.19	402.12	402.12	402.12

Steel Bars ...

Top.Sup.	2T12		2T16	2T16		2T20	2T20		2T20
Top.Sup.									
Bot.Bars	2T12			2T12			2T16		
Bot.Bars									
Bot.Sup.									
Side Bar									

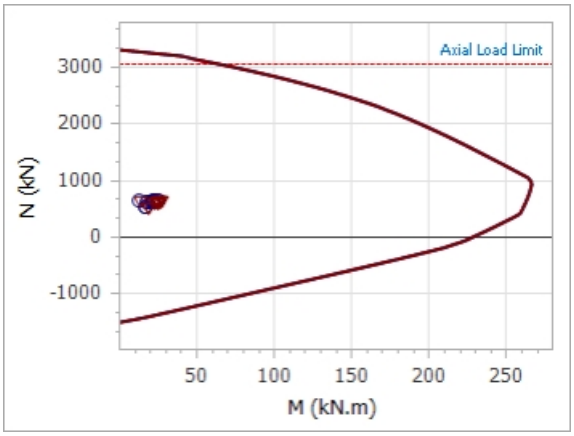
COLUMN

Column Reinforcement Design

1C1 (B-2) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	636.8, 636.8	22.5, -10.8	18.7, -9.0
2	616.8, 616.8	22.3, -10.7	18.4, -8.9
3	537.1, 537.1	16.9, -8.1	14.1, -6.8
4	623.2, 623.2	22.7, -11.0	15.4, -2.6
5	650.4, 650.4	22.4, -10.6	22.1, -15.4
6	630.3, 630.3	20.3, -6.5	18.8, -9.1
7	643.3, 643.3	24.8, -15.2	18.6, -8.9

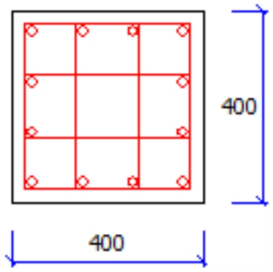


Critical Loading: 1 - (G*F)

		min	Design	
N	678.6	-	678.6	kN
M_{11}	16.3	13.6	19.1	kN.m
M_{22}	13.6	13.6	16.5	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 224.1 mm / 12.74 °

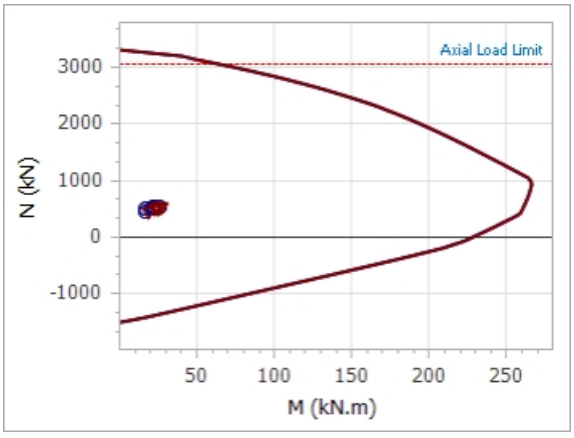


$V_{Ed(1/2)}$ =	10.3 / 10.4	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.07 / 0.07	N/mm²	λ_1/Lim_1 =	14.8 < 66.0	√	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	14.7 < 64.6	√			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

2C1 (B-2) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	523.2, 523.2	22.4, -22.5	18.5, -18.7
2	503.6, 503.6	22.2, -22.3	18.3, -18.4
3	443.8, 443.8	16.7, -16.9	13.9, -14.1
4	513.4, 513.4	22.4, -22.5	14.2, -14.4
5	533.1, 533.1	22.3, -22.5	22.9, -23.1
6	518.5, 518.5	20.0, -20.3	18.6, -18.7
7	528.0, 528.0	24.8, -24.7	18.5, -18.7

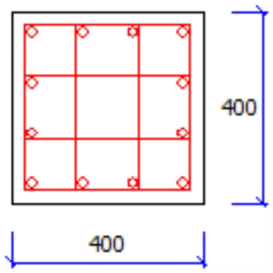


Critical Loading: 1 - (G*F)

		min	Design	
N	566.4	-	566.4	kN
M_{11}	-24.2	-11.3	-27.1	kN.m
M_{22}	-20.1	-11.3	-23.0	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 220.1 mm / 17.46 °



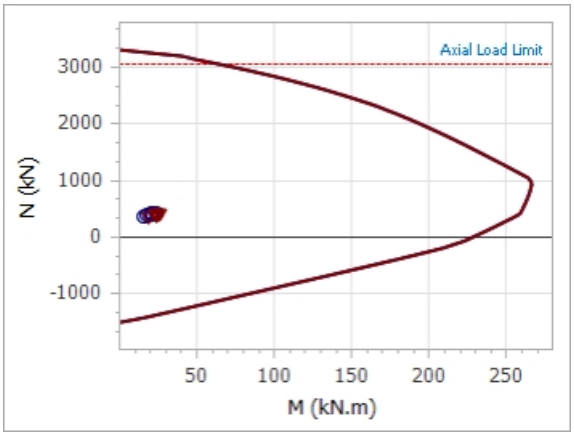
$V_{Ed(1/2)}$ =	15.9 / 17.2	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.10 / 0.10	N/mm²	λ_1/Lim_1 =	17.8 < 81.9	√	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	17.6 < 81.8	√			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

3C1 (B-2) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	410.1, 410.1	22.1, -22.4	18.2, -18.5
2	390.5, 390.5	21.9, -22.2	18.0, -18.3
3	350.7, 350.7	16.5, -16.7	13.6, -13.9
4	403.8, 403.8	22.2, -22.4	14.5, -15.4
5	416.3, 416.3	22.1, -22.4	21.9, -21.6
6	407.0, 407.0	20.1, -20.7	18.2, -18.5
7	413.1, 413.1	24.2, -24.0	18.2, -18.5

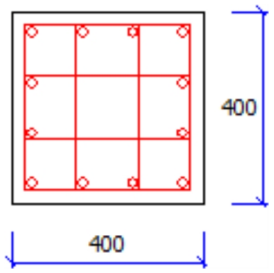


Critical Loading: 1 - (G*F)

		min	Design	
N	448.0	-	448.0	kN
M_{11}	23.2	9.0	25.2	kN.m
M_{22}	18.9	9.0	20.8	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 185.1 mm / 15.88 °

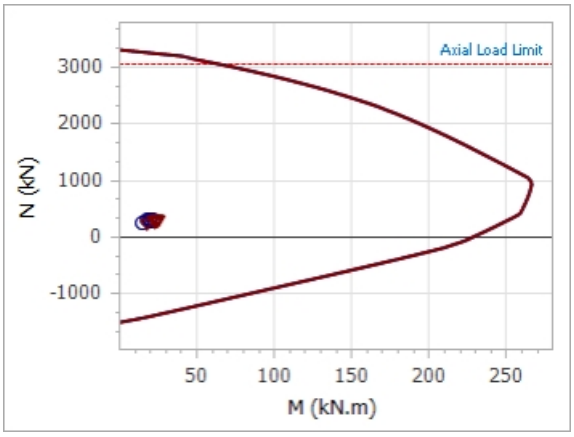


$V_{Ed(1/2)}$ =	14.7 / 16.7 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.10 / 0.10 N/mm²	λ_1/Lim_1 =	15.4 < 93.3 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	1.09 / 1.08 N/mm²	λ_2/Lim_2 =	15.4 < 93.9 ✓		
$v_{Rd\ Max}$ =	4.72 / 4.71 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	
Links = R10-300					

4C1 (B-2) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	297.5, 297.5	21.9, -22.1	17.9, -18.2
2	278.0, 278.0	21.7, -21.9	17.8, -18.0
3	258.2, 258.2	16.3, -16.5	13.3, -13.6
4	294.1, 294.1	21.9, -22.2	15.1, -16.1
5	300.9, 300.9	21.9, -22.1	20.7, -20.3
6	295.7, 295.7	20.3, -21.0	17.9, -18.2
7	299.2, 299.2	23.5, -23.3	17.9, -18.2

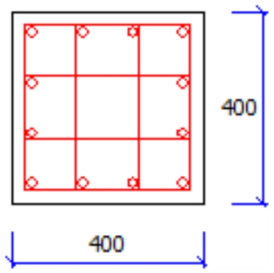


Critical Loading: 1 - (G*F)

		min	Design	
N	327.9	-	327.9	kN
M_{11}	-24.6	-6.6	-26.1	kN.m
M_{22}	19.8	6.6	21.2	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 175.4 mm / 19.60 °



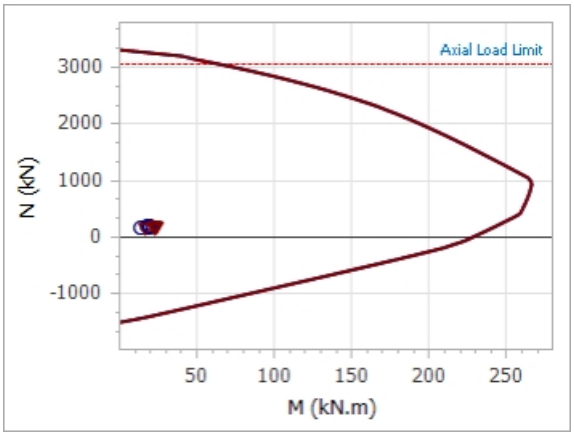
$V_{Ed(1/2)}$ =	14.8 / 17.3 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.10 / 0.10 N/mm²	λ_1/Lim_1 =	15.4 < 109.5 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.97 / 0.97 N/mm²	λ_2/Lim_2 =	15.4 < 109.9 ✓		
$v_{Rd\ Max}$ =	4.53 / 4.53 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	

Links = R10-300		

5C1 (B-2) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	185.6, 185.6	21.6, -21.9	17.5, -17.9
2	166.1, 166.1	21.4, -21.7	17.5, -17.8
3	166.3, 166.3	16.1, -16.3	13.0, -13.3
4	184.1, 184.1	21.6, -21.9	15.7, -16.7
5	187.0, 187.0	21.6, -21.9	19.4, -19.1
6	184.8, 184.8	20.5, -21.2	17.5, -17.9
7	186.4, 186.4	22.7, -22.6	17.5, -17.9

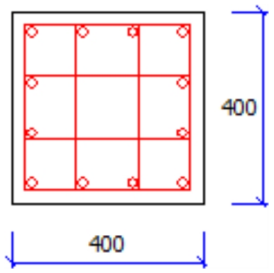


Critical Loading: 1 - (G*F)

		min	Design	
N	206.8	-	206.8	kN
M_{11}	-24.8	-4.1	-25.7	kN.m
M_{22}	-19.5	-4.1	-20.4	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 128.6 mm / 15.81 °

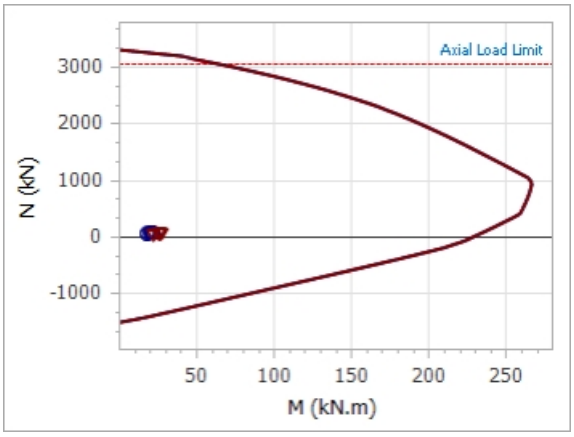


$V_{Ed(1/2)}$ =	13.8 / 16.9 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.09 / 0.09 N/mm²	λ_1/Lim_1 =	15.4 < 137.1 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.86 / 0.86 N/mm²	λ_2/Lim_2 =	15.4 < 136.8 ✓		
$v_{Rd\ Max}$ =	4.34 / 4.34 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	
Links = R10-300					

6C1 (B-2) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	74.3, 74.3	25.7, -21.6	20.3, -17.5
2	54.9, 54.9	16.6, -21.4	13.2, -17.5
3	75.0, 75.0	25.7, -16.1	20.3, -13.0
4	73.9, 73.9	25.7, -21.6	19.4, -17.2
5	74.8, 74.8	25.6, -21.6	21.1, -17.9
6	74.1, 74.1	25.1, -21.4	20.3, -17.5
7	74.6, 74.6	26.2, -21.8	20.3, -17.5

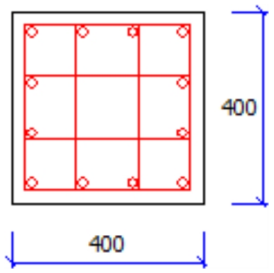


Critical Loading: 7 - (G+Q-Nx)

		min	Design	
N	84.8	-	84.8	kN
M_{11}	32.2	1.7	32.6	kN.m
M_{22}	25.0	1.7	25.4	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 146.5 mm / 28.55 °



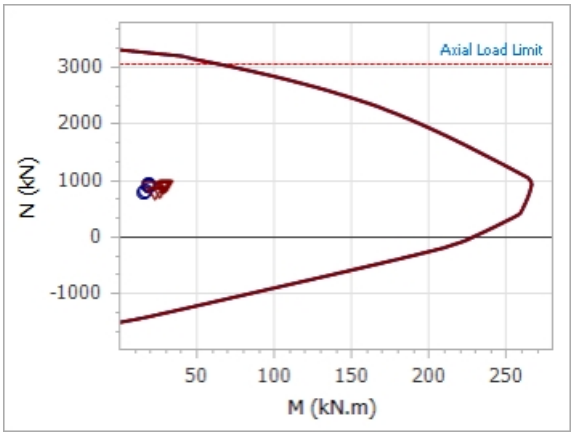
$V_{Ed(1/2)}$ =	16.0 / 19.9 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.11 / 0.11 N/mm²	λ_1/Lim_1 =	15.4 < 227.0 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.74 / 0.74 N/mm²	λ_2/Lim_2 =	15.4 < 225.4 ✓		
$v_{Rd\ Max}$ =	4.16 / 4.16 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	

Links = R10-300		

1C2 (B-3) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	914.6, 914.6	24.8, -11.9	-11.8, 5.7
2	829.2, 829.2	22.4, -10.8	-12.6, 6.1
3	794.3, 794.3	19.1, -9.2	-8.1, 3.9
4	918.3, 918.3	24.9, -12.0	-15.1, 12.0
5	911.0, 911.0	24.7, -11.8	-8.4, -0.7
6	911.2, 911.2	23.2, -7.9	-11.6, 5.5
7	918.1, 918.1	26.4, -16.0	-11.9, 5.8

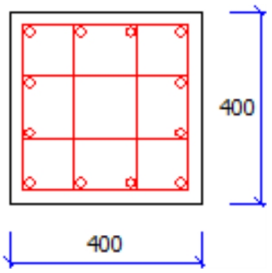


Critical Loading: 1 - (G*F)

		min	Design	
N	853.9	-	853.9	kN
M_{11}	20.9	17.1	24.5	kN.m
M_{22}	-11.3	-17.1	-17.1	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 291.7 mm / 18.54 °

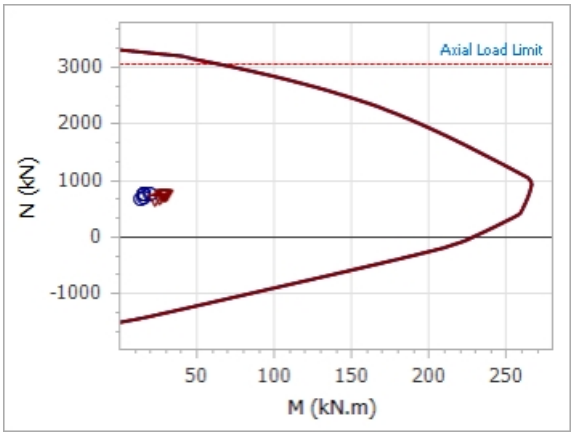


$V_{Ed(1/2)}$ =	8.5 / 12.4	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.06 / 0.06	N/mm²	λ_1/Lim_1 =	13.0 < 56.2	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	15.0 < 57.5	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

2C2 (B-3) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	762.3, 762.3	24.5, -24.8	-11.9, 11.8
2	689.3, 689.3	22.2, -22.4	-12.7, 12.6
3	662.6, 662.6	18.9, -19.1	-8.2, 8.1
4	764.7, 764.7	24.7, -24.9	-16.2, 16.1
5	759.8, 759.8	24.4, -24.7	-7.5, 7.4
6	759.6, 759.6	22.8, -23.3	-11.8, 11.8
7	764.9, 764.9	26.2, -26.3	-11.9, 11.7

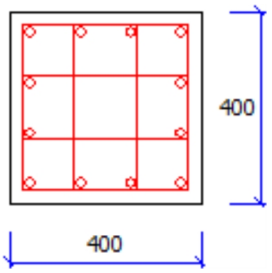


Critical Loading: 1 - (G*F)

		min	Design	
N	705.2	-	705.2	kN
M_{11}	-30.0	-14.1	-33.6	kN.m
M_{22}	14.4	14.1	17.6	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 288.2 mm / 24.68 °



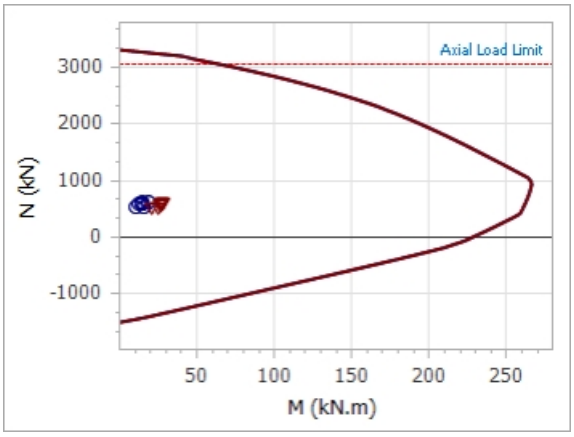
$V_{Ed(1/2)}$ =	12.1 / 20.3	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.08 / 0.08	N/mm²	λ_1/Lim_1 =	15.6 < 72.9	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	17.6 < 72.9	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

3C2 (B-3) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	603.6, 603.6	24.3, -24.5	-11.8, 11.9
2	543.1, 543.1	22.0, -22.2	-12.6, 12.7
3	525.1, 525.1	18.7, -18.9	-8.3, 8.2
4	604.9, 604.9	24.4, -24.6	-15.5, 15.0
5	602.3, 602.3	24.2, -24.4	-8.1, 8.8
6	601.7, 601.7	22.8, -23.4	-11.8, 11.9
7	605.4, 605.4	25.8, -25.6	-11.8, 11.9

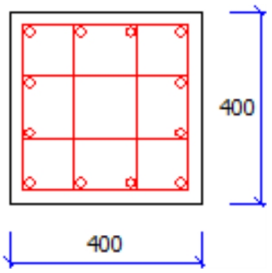


Critical Loading: 1 - (G*F)

		min	Design	
N	553.4	-	553.4	kN
M_{11}	27.1	11.1	29.5	kN.m
M_{22}	-11.7	-11.1	-14.2	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 239.6 mm / 22.02 °

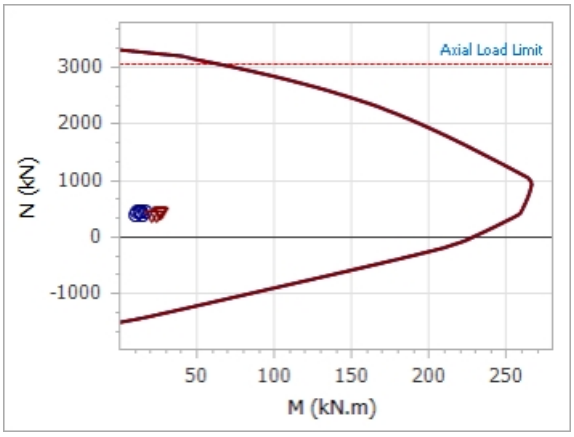


$V_{Ed(1/2)}$ =	10.1 / 18.9	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.07 / 0.07	N/mm²	λ_1/Lim_1 =	15.4 < 84.5	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	15.4 < 84.4	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

4C2 (B-3) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	443.4, 443.4	24.0, -24.3	-11.8, 11.8
2	395.4, 395.4	21.8, -22.0	-12.5, 12.6
3	386.3, 386.3	18.5, -18.7	-8.3, 8.3
4	444.0, 444.0	24.1, -24.4	-14.6, 14.0
5	442.9, 442.9	24.0, -24.2	-9.0, 9.7
6	442.3, 442.3	22.8, -23.5	-11.8, 11.8
7	444.6, 444.6	25.3, -25.0	-11.8, 11.8

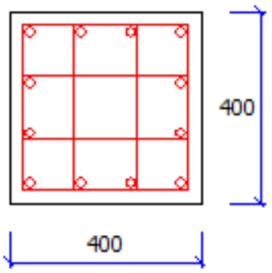


Critical Loading: 1 - (G*F)

		min	Design	
N	402.8	-	402.8	kN
M_{11}	28.5	8.1	30.3	kN.m
M_{22}	11.5	8.1	13.3	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 148.6 mm / 10.20 °



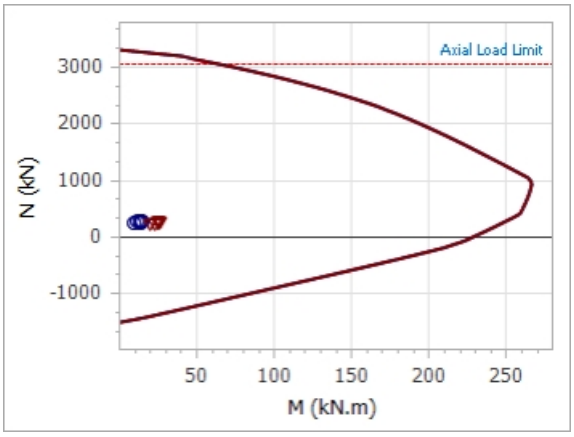
$V_{Ed(1/2)}$ =	9.3 / 19.6	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.06 / 0.06	N/mm²	λ_1/Lim_1 =	15.4 < 98.8	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.04 / 1.04	N/mm²	λ_2/Lim_2 =	15.4 < 99.1	✓			
$v_{Rd\ Max}$ =	4.64 / 4.64	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

5C2 (B-3) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	281.7, 281.7	23.8, -24.0	-11.7, 11.8
2	246.2, 246.2	21.6, -21.8	-12.4, 12.5
3	246.4, 246.4	18.2, -18.5	-8.3, 8.3
4	281.8, 281.8	23.8, -24.1	-13.6, 13.0
5	281.6, 281.6	23.7, -24.0	-9.8, 10.6
6	281.0, 281.0	22.9, -23.6	-11.7, 11.8
7	282.3, 282.3	24.7, -24.5	-11.7, 11.8

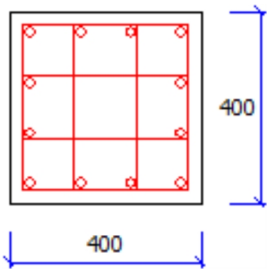


Critical Loading: 1 - (G*F)

		min	Design	
N	253.1	-	253.1	kN
M_{11}	-27.8	-5.1	-29.0	kN.m
M_{22}	11.0	5.1	12.1	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 175.7 mm / 24.11 °

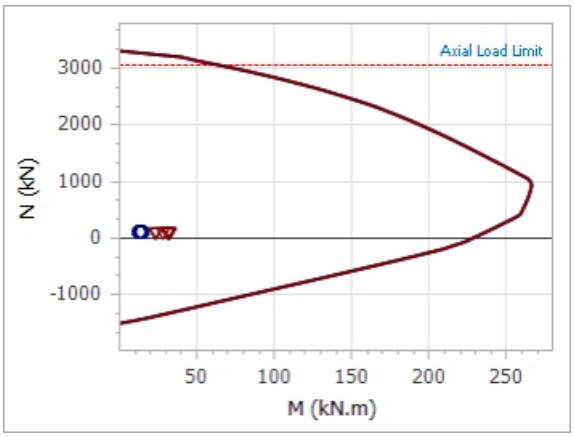


$V_{Ed(1/2)}$ =	8.1 / 18.5 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.05 / 0.05 N/mm²	λ_1/Lim_1 =	15.4 < 124.0 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.87 / 0.90 N/mm²	λ_2/Lim_2 =	15.4 < 122.6 ✓		
$v_{Rd\ Max}$ =	4.36 / 4.41 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	
Links = R10-300					

6C2 (B-3) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	118.3, 118.3	31.2, -23.8	-12.7, 11.7
2	95.2, 95.2	22.4, -21.6	-7.4, 12.4
3	105.3, 105.3	28.1, -18.2	-13.8, 8.3
4	118.2, 118.2	31.3, -23.8	-13.6, 12.0
5	118.3, 118.3	31.2, -23.7	-11.9, 11.4
6	118.0, 118.0	30.7, -23.7	-12.7, 11.7
7	118.5, 118.5	31.7, -23.9	-12.7, 11.7

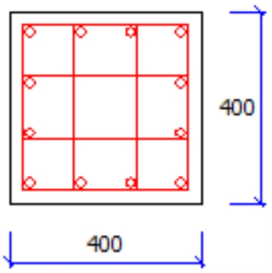


Critical Loading: 7 - (G+Q-Nx)

		min	Design	
N	104.2	-	104.2	kN
M_{11}	40.5	2.1	41.0	kN.m
M_{22}	-11.7	-2.1	-12.1	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 543.5 mm / 81.32 °



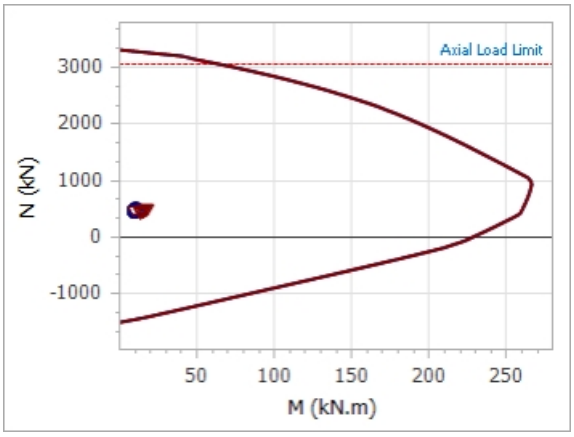
$V_{Ed(1/2)}$ =	8.9 / 24.2 kN	Short Column...		As (Req):	%0.21 329.98 mm²
v_{Ed} =	0.06 / 0.06 N/mm²	λ_1/Lim_1 =	15.4 < 210.0 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.74 / 0.76 N/mm²	λ_2/Lim_2 =	15.4 < 198.7 ✓		
$v_{Rd\ Max}$ =	4.16 / 4.19 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	

Links = R10-300		

1C3 (A-4) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	489.6, 489.6	13.3, -6.4	-0.5, 0.2
2	444.7, 444.7	11.4, -5.5	0.5, -0.2
3	453.3, 453.3	11.3, -5.4	-1.4, 0.7
4	489.4, 489.4	13.3, -6.4	-4.1, 6.8
5	489.8, 489.8	13.3, -6.4	3.1, -6.3
6	479.5, 479.5	11.3, -2.2	-0.4, 0.1
7	499.7, 499.7	15.2, -10.6	-0.6, 0.4

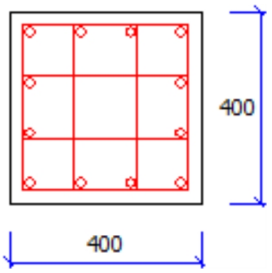


Critical Loading: 1 - (G*F)

		min	Design	
N	631.7	-	631.7	kN
M_{11}	11.0	12.6	13.6	kN.m
M_{22}	-1.1	-12.6	-12.6	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 203.1 mm / 10.49 °

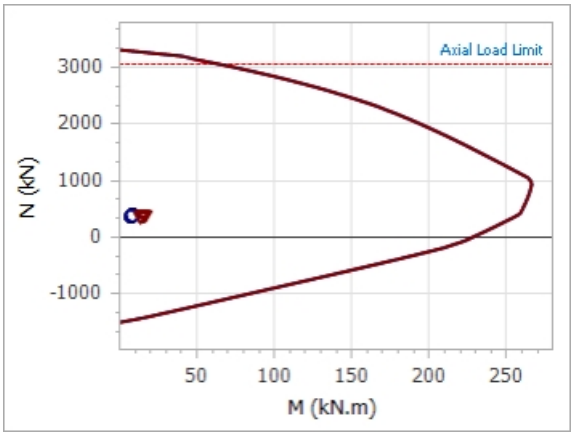


$V_{Ed(1/2)}$ =	3.8 / 7.7	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.03 / 0.03	N/mm²	λ_1/Lim_1 =	15.3 < 20.5	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	14.0 < 68.1	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

2C3 (A-4) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	397.2, 397.2	12.8, -13.3	-0.4, 0.5
2	360.4, 360.4	11.0, -11.4	0.5, -0.5
3	368.6, 368.6	10.8, -11.3	-1.3, 1.4
4	397.1, 397.1	12.8, -13.3	-5.1, 5.2
5	397.3, 397.3	12.8, -13.3	4.3, -4.2
6	390.2, 390.2	10.7, -11.4	-0.4, 0.6
7	404.1, 404.1	14.9, -15.2	-0.4, 0.5

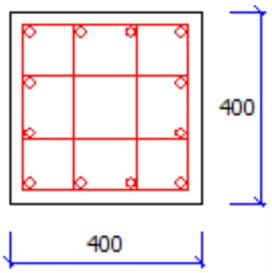


Critical Loading: 1 - (G*F)

		min	Design	
N	531.7	-	531.7	kN
M_{11}	-16.7	-10.6	-19.3	kN.m
M_{22}	-1.1	-10.6	-10.6	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 170.3 mm / 8.35 °



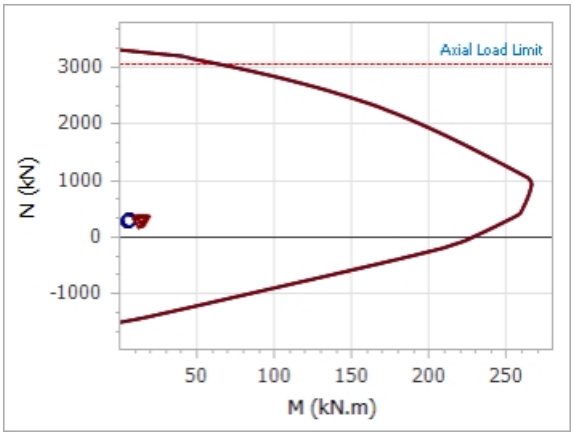
$V_{Ed(1/2)}$ =	3.7 / 12.1	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.02 / 0.02	N/mm²	λ_1/Lim_1 =	18.0 < 22.4	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.16 / 1.17	N/mm²	λ_2/Lim_2 =	16.5 < 84.5	✓			
$v_{Rd\ Max}$ =	4.84 / 4.85	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

3C3 (A-4) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	305.2, 305.2	12.2, -12.8	-0.3, 0.4
2	276.4, 276.4	10.6, -11.0	0.6, -0.5
3	283.6, 283.6	10.4, -10.8	-1.1, 1.3
4	305.1, 305.1	12.2, -12.8	-4.4, 3.9
5	305.2, 305.2	12.2, -12.8	3.8, -3.1
6	300.9, 300.9	10.5, -11.4	-0.3, 0.5
7	309.4, 309.4	14.0, -14.2	-0.2, 0.3

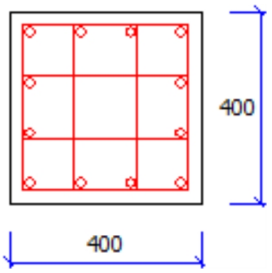


Critical Loading: 1 - (G*F)

		min	Design	
N	422.1	-	422.1	kN
M_{11}	16.5	8.4	18.3	kN.m
M_{22}	-0.3	-8.4	-8.4	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 170.8 mm / 14.08 °

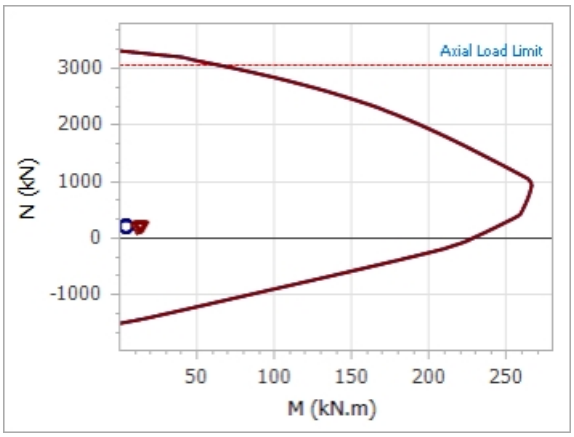


$V_{Ed(1/2)}$ =	2.7 / 12.0	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.02 / 0.02	N/mm²	λ_1/Lim_1 =	15.4 < 25.1	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.06 / 1.06	N/mm²	λ_2/Lim_2 =	15.4 < 96.5	✓			
$v_{Rd\ Max}$ =	4.67 / 4.68	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

4C3 (A-4) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	215.7, 215.7	11.6, -12.2	-0.1, 0.3
2	195.0, 195.0	10.1, -10.6	0.6, -0.6
3	200.5, 200.5	9.9, -10.4	-0.9, 1.1
4	215.7, 215.7	11.7, -12.2	-3.3, 2.8
5	215.7, 215.7	11.6, -12.2	3.1, -2.3
6	213.5, 213.5	10.3, -11.3	-0.2, 0.3
7	217.9, 217.9	13.0, -13.1	-0.1, 0.2

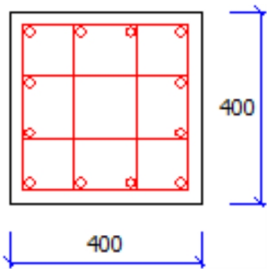


Critical Loading: 1 - (G*F)

		min	Design	
N	309.3	-	309.3	kN
M_{11}	17.9	6.2	19.3	kN.m
M_{22}	-0.1	-6.2	-6.2	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 147.3 mm / 14.61 °



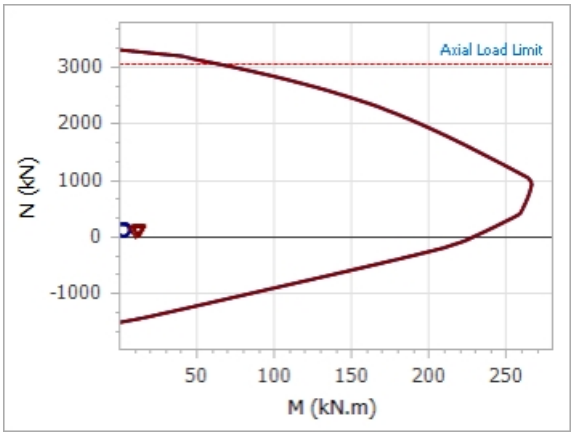
$V_{Ed(1/2)}$ =	2.0 / 12.6	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.01 / 0.01	N/mm²	λ_1/Lim_1 =	15.4 < 29.3	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	0.95 / 0.95	N/mm²	λ_2/Lim_2 =	15.4 < 112.9	✓			
$v_{Rd\ Max}$ =	4.50 / 4.50	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

5C3 (A-4) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	129.0, 129.0	11.0, -11.6	0.0, 0.1
2	116.3, 116.3	9.6, -10.1	0.7, -0.6
3	119.4, 119.4	9.3, -9.9	-0.7, 0.9
4	129.0, 129.0	11.0, -11.6	-2.2, 1.7
5	128.9, 128.9	11.0, -11.6	2.2, -1.4
6	128.1, 128.1	10.1, -11.2	-0.1, 0.2
7	129.8, 129.8	12.0, -12.1	0.1, 0.0

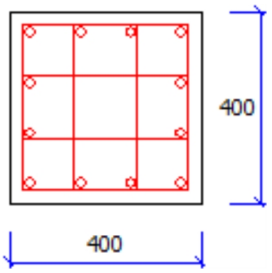


Critical Loading: 1 - (G*F)

		min	Design	
N	194.5	-	194.5	kN
M_{11}	-18.0	-3.9	-18.9	kN.m
M_{22}	-0.4	-3.9	-3.9	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 68.8 mm / 2.94 °

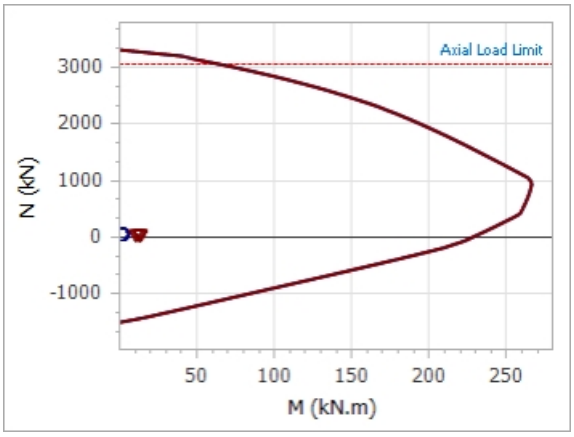


$V_{Ed(1/2)}$ =	1.5 / 12.2	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.01 / 0.01	N/mm²	λ_1/Lim_1 =	15.4 < 37.0	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	0.84 / 0.84	N/mm²	λ_2/Lim_2 =	15.4 < 140.5	✓			
$v_{Rd\ Max}$ =	4.32 / 4.32	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

6C3 (A-4) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	45.2, 45.2	12.9, -11.0	0.5, 0.0
2	40.5, 40.5	10.3, -9.6	-1.0, -0.7
3	40.3, 40.3	10.7, -9.3	1.9, 0.7
4	45.2, 45.2	12.9, -11.0	-0.8, 0.7
5	45.1, 45.1	12.9, -11.0	1.9, -0.7
6	45.0, 45.0	12.6, -11.0	0.4, 0.1
7	45.3, 45.3	13.3, -11.1	0.7, -0.1

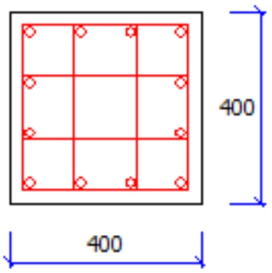


Critical Loading: 1 - (G*F)

		min	Design	
N	77.5	-	77.5	kN
M_{11}	25.4	1.6	25.8	kN.m
M_{22}	0.1	1.6	1.6	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 36.3 mm / 1.08 °



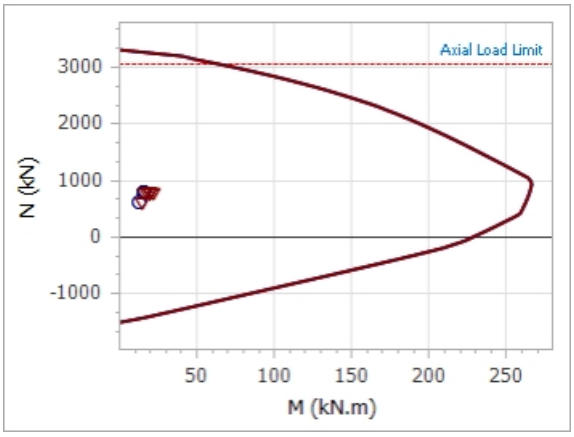
$V_{Ed(1/2)}$ =	0.7 / 15.5	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.00 / 0.00	N/mm²	λ_1/Lim_1 =	15.4 < 58.6	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	0.73 / 0.73	N/mm²	λ_2/Lim_2 =	15.4 < 210.5	✓			
$v_{Rd\ Max}$ =	4.15 / 4.15	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

1C4 (B-5) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	783.0, 783.0	16.4, -7.9	10.7, -5.1
2	774.4, 774.4	15.9, -7.7	10.9, -5.3
3	622.8, 622.8	11.5, -5.5	7.9, -3.8
4	782.7, 782.7	16.1, -7.7	8.3, 0.8
5	783.2, 783.2	16.6, -8.0	13.0, -11.0
6	778.0, 778.0	14.6, -3.8	10.7, -5.2
7	788.0, 788.0	18.2, -12.0	10.6, -5.0

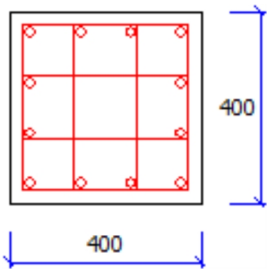


Critical Loading: 1 - (G*F)

		min	Design	
N	751.8	-	751.8	kN
M_{11}	12.1	15.0	15.5	kN.m
M_{22}	12.4	15.0	15.2	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 237.3 mm / 11.87 °

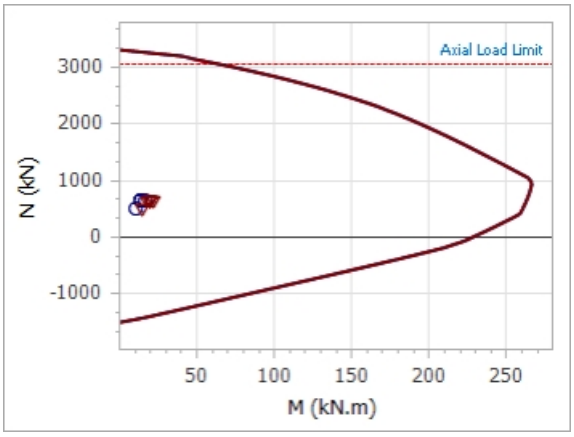


$V_{Ed(1/2)}$ =	9.2 / 8.2	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.06 / 0.06	N/mm²	λ_1/Lim_1 =	13.2 < 63.0	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	15.6 < 62.7	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

2C4 (B-5) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	650.8, 650.8	15.8, -16.4	10.6, -10.7
2	642.4, 642.4	15.5, -15.9	10.9, -10.9
3	517.9, 517.9	10.9, -11.5	8.0, -7.9
4	650.7, 650.7	15.5, -16.1	7.5, -7.7
5	650.9, 650.9	16.1, -16.7	13.7, -13.7
6	647.1, 647.1	13.8, -14.6	10.6, -10.6
7	654.5, 654.5	17.8, -18.1	10.6, -10.7

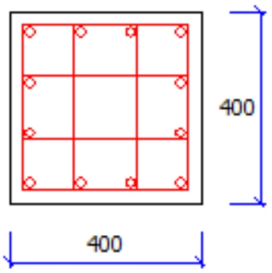


Critical Loading: 1 - (G*F)

		min	Design	
N	622.9	-	622.9	kN
M_{11}	-18.2	-12.5	-21.5	kN.m
M_{22}	-17.7	-12.5	-20.5	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 224.8 mm / 15.65 °



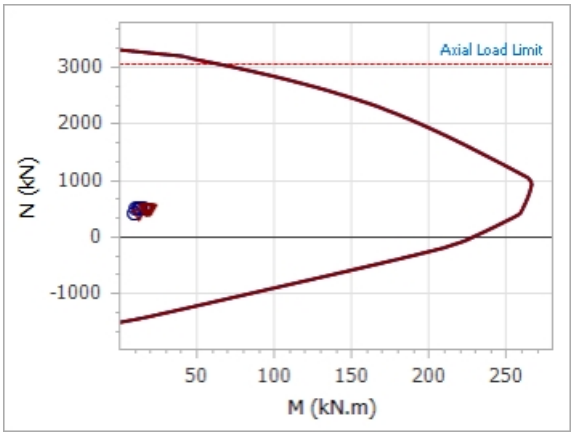
$V_{Ed(1/2)}$ =	13.2 / 13.0	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.09 / 0.09	N/mm²	λ_1/Lim_1 =	15.5 < 77.0	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	18.3 < 78.1	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

3C4 (B-5) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	514.3, 514.3	15.4, -15.8	10.6, -10.6
2	505.7, 505.7	15.2, -15.5	10.7, -10.9
3	409.2, 409.2	10.5, -10.9	8.0, -8.0
4	514.2, 514.2	15.2, -15.5	7.9, -8.6
5	514.3, 514.3	15.7, -16.1	13.3, -12.7
6	511.8, 511.8	13.7, -14.5	10.6, -10.6
7	516.8, 516.8	17.2, -17.1	10.6, -10.7

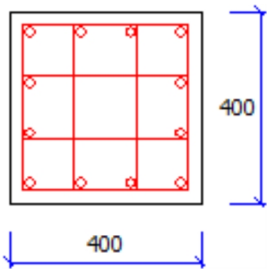


Critical Loading: 1 - (G*F)

		min	Design	
N	489.4	-	489.4	kN
M_{11}	17.6	9.8	19.8	kN.m
M_{22}	14.5	9.8	16.7	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 180.7 mm / 12.87 °

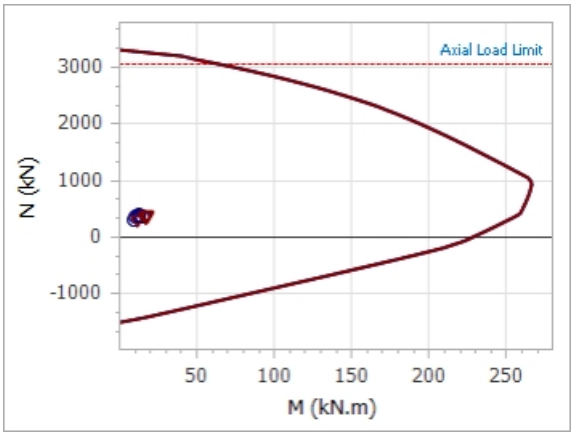


$V_{Ed(1/2)}$ =	11.2 / 12.7 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.07 / 0.07 N/mm²	λ_1/Lim_1 =	15.4 < 89.8 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	1.12 / 1.12 N/mm²	λ_2/Lim_2 =	15.4 < 89.7 ✓		
$v_{Rd\ Max}$ =	4.77 / 4.78 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	
Links = R10-300					

4C4 (B-5) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	376.8, 376.8	15.1, -15.4	10.5, -10.6
2	367.8, 367.8	15.0, -15.2	10.6, -10.7
3	300.1, 300.1	10.2, -10.5	8.0, -8.0
4	376.8, 376.8	14.9, -15.2	8.4, -9.2
5	376.8, 376.8	15.3, -15.7	12.6, -11.9
6	375.3, 375.3	13.7, -14.5	10.5, -10.5
7	378.3, 378.3	16.4, -16.4	10.5, -10.6

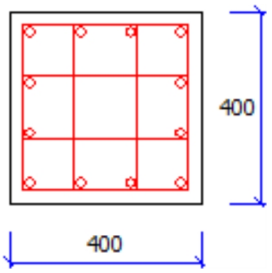


Critical Loading: 1 - (G*F)

		min	Design	
N	356.2	-	356.2	kN
M_{11}	18.9	7.1	20.5	kN.m
M_{22}	-14.7	-7.1	-16.3	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 162.7 mm / 15.57 °



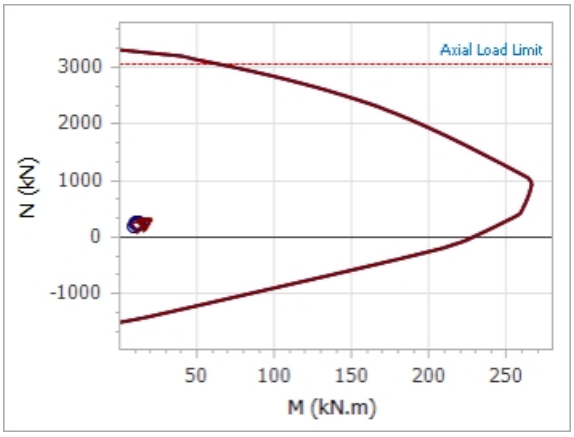
$V_{Ed(1/2)}$ =	10.5 / 13.3 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.07 / 0.07 N/mm²	λ_1/Lim_1 =	15.4 < 105.2 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.95 / 1.00 N/mm²	λ_2/Lim_2 =	15.4 < 105.2 ✓		
$v_{Rd\ Max}$ =	4.50 / 4.57 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	

Links = R10-300		

5C4 (B-5) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	238.4, 238.4	14.7, -15.1	10.4, -10.5
2	228.4, 228.4	14.8, -15.0	10.5, -10.6
3	190.6, 190.6	9.8, -10.2	8.0, -8.0
4	238.4, 238.4	14.6, -14.9	8.9, -9.8
5	238.3, 238.3	14.8, -15.2	11.9, -11.2
6	237.6, 237.6	13.7, -14.6	10.4, -10.5
7	239.1, 239.1	15.7, -15.6	10.5, -10.6

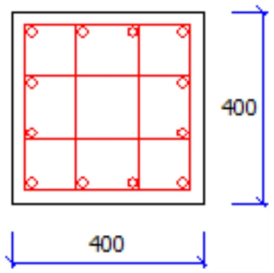


Critical Loading: 1 - (G*F)

		min	Design	
N	223.2	-	223.2	kN
M_{11}	-18.8	-4.5	-19.8	kN.m
M_{22}	-14.0	-4.5	-15.0	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 112.9 mm / 11.45 °

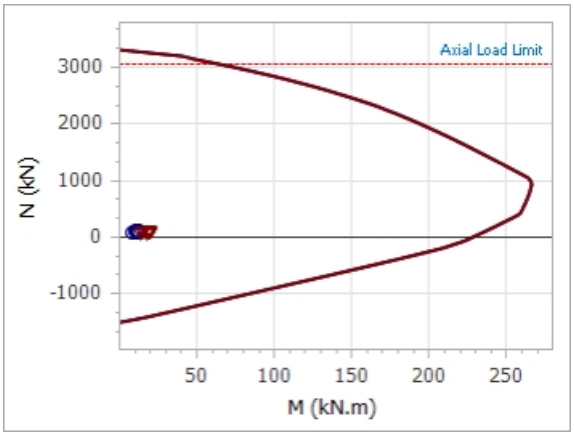


$V_{Ed(1/2)}$ =	9.9 / 12.8	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.07 / 0.07	N/mm²	λ_1/Lim_1 =	15.4 < 131.4	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	0.84 / 0.87	N/mm²	λ_2/Lim_2 =	15.4 < 131.2	✓			
$v_{Rd\ Max}$ =	4.32 / 4.37	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

6C4 (B-5) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	98.8, 98.8	18.9, -14.7	11.0, -10.4
2	87.6, 87.6	16.9, -14.8	12.1, -10.5
3	80.6, 80.6	13.6, -9.8	6.0, -8.0
4	98.8, 98.8	18.8, -14.6	10.3, -10.3
5	98.8, 98.8	19.0, -14.8	11.7, -10.6
6	98.5, 98.5	18.4, -14.5	10.9, -10.4
7	99.0, 99.0	19.4, -14.9	11.0, -10.5

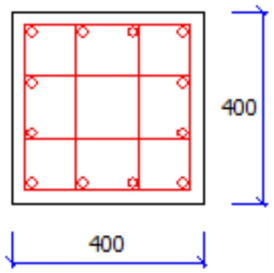


Critical Loading: 7 - (G+Q-Nx)

		min	Design	
N	90.3	-	90.3	kN
M_{11}	27.5	1.8	27.9	kN.m
M_{22}	16.8	1.8	17.2	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 102.1 mm / 16.97 °



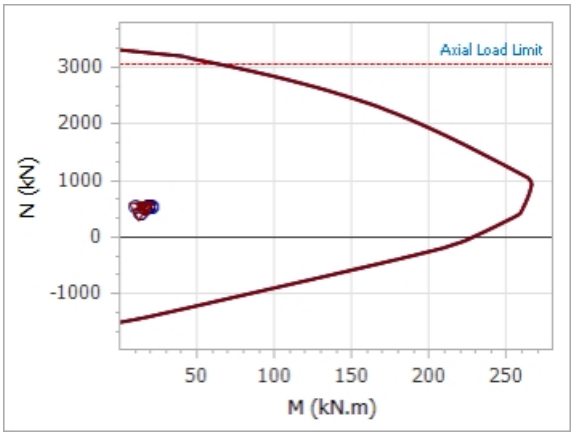
$V_{Ed(1/2)}$ =	11.7 / 16.5	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.08 / 0.08	N/mm²	λ_1/Lim_1 =	15.4 < 221.8	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	0.73 / 0.75	N/mm²	λ_2/Lim_2 =	15.4 < 213.8	✓			
$v_{Rd\ Max}$ =	4.14 / 4.17	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

1C5 (B-6) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	526.7, 526.7	13.9, -6.7	-16.2, 7.8
2	528.6, 528.6	13.9, -6.7	-16.2, 7.8
3	430.0, 430.0	10.4, -5.0	-12.1, 5.8
4	536.4, 536.4	13.8, -6.5	-18.6, 13.7
5	517.0, 517.0	14.0, -6.8	-13.9, 1.9
6	520.1, 520.1	12.1, -2.6	-16.2, 7.7
7	533.3, 533.3	15.7, -10.7	-16.3, 7.9

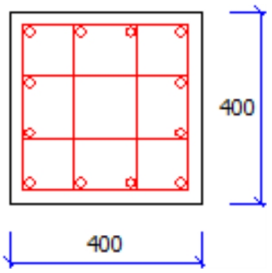


Critical Loading: 1 - (G*F)

		min	Design	
N	566.4	-	566.4	kN
M_{11}	10.9	11.3	13.4	kN.m
M_{22}	-13.0	-11.3	-15.5	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 187.3 mm / 10.40 °

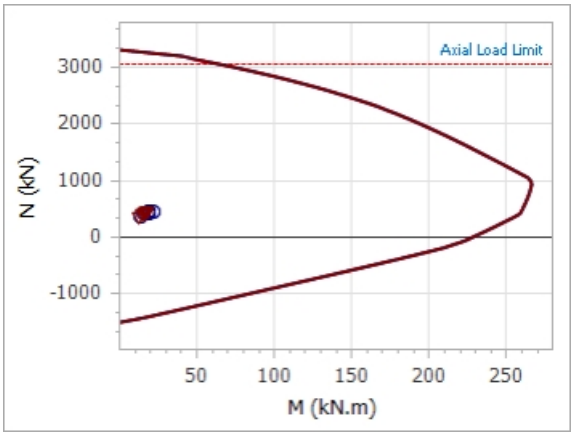


$V_{Ed(1/2)}$ =	8.9 / 7.5	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.06 / 0.06	N/mm²	λ_1/Lim_1 =	15.5 < 68.2	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.18 / 1.18	N/mm²	λ_2/Lim_2 =	15.6 < 72.2	✓			
$v_{Rd\ Max}$ =	4.86 / 4.86	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

2C5 (B-6) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	435.4, 435.4	13.8, -13.9	-16.2, 16.2
2	436.9, 436.9	13.8, -13.9	-16.2, 16.2
3	355.0, 355.0	10.3, -10.4	-12.2, 12.1
4	442.5, 442.5	13.8, -13.9	-19.3, 19.2
5	428.3, 428.3	13.8, -13.9	-13.2, 13.3
6	430.7, 430.7	11.8, -12.1	-16.2, 16.3
7	440.0, 440.0	15.7, -15.7	-16.2, 16.2

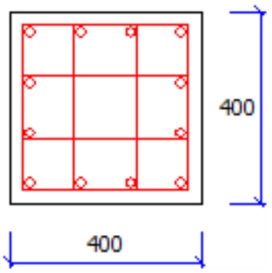


Critical Loading: 1 - (G*F)

		min	Design	
N	471.4	-	471.4	kN
M_{11}	-16.1	-9.4	-18.6	kN.m
M_{22}	17.3	9.4	19.7	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 183.0 mm / 14.27 °



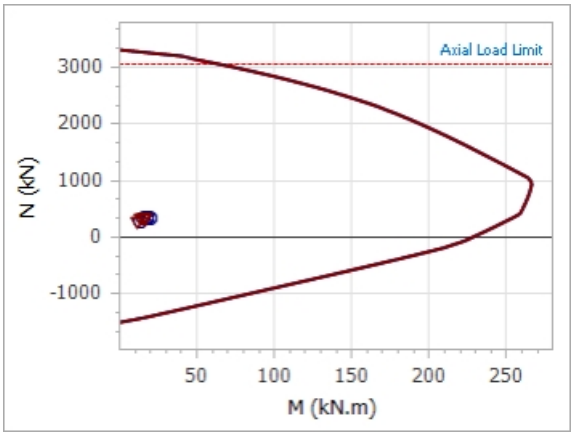
$V_{Ed(1/2)}$ =	13.2 / 11.5	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.09 / 0.09	N/mm²	λ_1/Lim_1 =	18.2 < 90.1	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.11 / 1.11	N/mm²	λ_2/Lim_2 =	18.3 < 89.1	✓			
$v_{Rd\ Max}$ =	4.75 / 4.75	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

3C5 (B-6) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	341.1, 341.1	13.6, -13.8	-16.0, 16.2
2	342.4, 342.4	13.6, -13.8	-16.0, 16.2
3	277.1, 277.1	10.1, -10.3	-12.0, 12.2
4	345.6, 345.6	13.6, -13.8	-18.7, 18.3
5	336.5, 336.5	13.6, -13.8	-13.3, 14.2
6	338.1, 338.1	11.9, -12.5	-16.0, 16.3
7	344.0, 344.0	15.3, -15.1	-16.0, 16.2

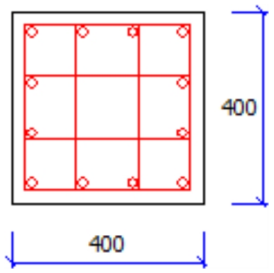


Critical Loading: 1 - (G*F)

		min	Design	
N	372.0	-	372.0	kN
M_{11}	14.7	7.4	16.4	kN.m
M_{22}	-16.1	-7.4	-17.7	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 151.9 mm / 12.48 °

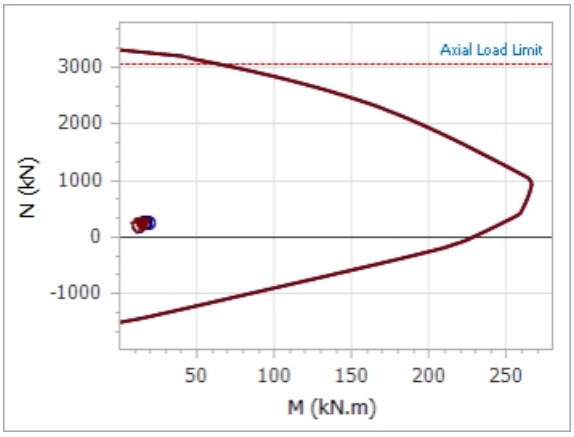


$V_{Ed(1/2)}$ =	12.2 / 10.8	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.08 / 0.08	N/mm²	λ_1/Lim_1 =	15.4 < 102.1	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	1.01 / 1.01	N/mm²	λ_2/Lim_2 =	15.4 < 102.9	✓			
$v_{Rd\ Max}$ =	4.60 / 4.60	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		
Links = R10-300								

4C5 (B-6) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	247.2, 247.2	13.4, -13.6	-15.8, 16.0
2	248.2, 248.2	13.5, -13.6	-15.8, 16.0
3	199.4, 199.4	10.0, -10.1	-11.9, 12.0
4	249.8, 249.8	13.4, -13.6	-17.9, 17.4
5	244.6, 244.6	13.4, -13.6	-13.7, 14.7
6	245.6, 245.6	12.1, -12.7	-15.8, 16.1
7	248.8, 248.8	14.7, -14.5	-15.8, 16.0

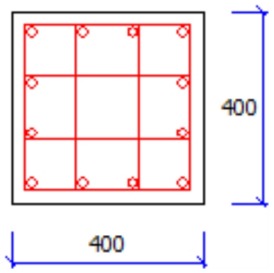


Critical Loading: 1 - (G*F)

		min	Design	
N	271.6	-	271.6	kN
M_{11}	-15.5	-5.4	-16.7	kN.m
M_{22}	-16.8	-5.4	-18.0	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 134.1 mm / 13.65 °



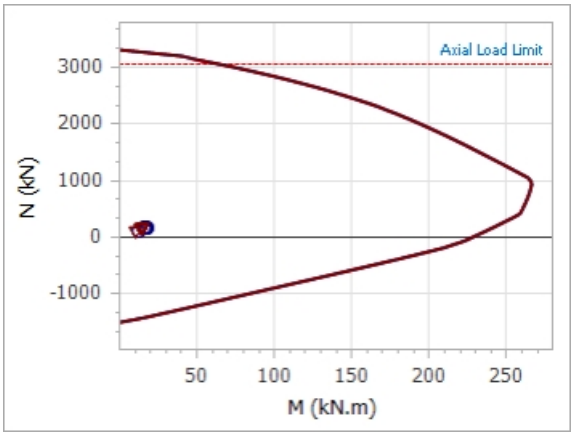
$V_{Ed(1/2)}$ =	12.3 / 11.0	kN	Short Column...			As (Req):	%0.20 (min)	320.00 mm²
v_{Ed} =	0.08 / 0.08	N/mm²	λ_1/Lim_1 =	15.4 < 120.3	✓	As (Sup):	%2.36	3769.91 mm²
v_{Rdc} =	0.92 / 0.92	N/mm²	λ_2/Lim_2 =	15.4 < 120.7	✓			
$v_{Rd\ Max}$ =	4.44 / 4.44	N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0	kN.m	**12T20**		

Links = R10-300		

5C5 (B-6) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	153.8, 153.8	13.2, -13.4	-15.6, 15.8
2	154.5, 154.5	13.3, -13.5	-15.5, 15.8
3	122.1, 122.1	9.8, -10.0	-11.7, 11.9
4	154.9, 154.9	13.2, -13.4	-17.0, 16.5
5	152.6, 152.6	13.2, -13.4	-14.1, 15.1
6	153.0, 153.0	12.3, -13.0	-15.6, 15.8
7	154.5, 154.5	14.2, -13.9	-15.5, 15.7

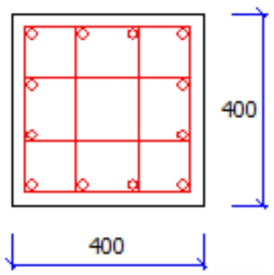


Critical Loading: 1 - (G*F)

		min	Design	
N	170.5	-	170.5	kN
M_{11}	-15.4	-3.4	-16.1	kN.m
M_{22}	16.5	3.4	17.3	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 105.6 mm / 12.46 °

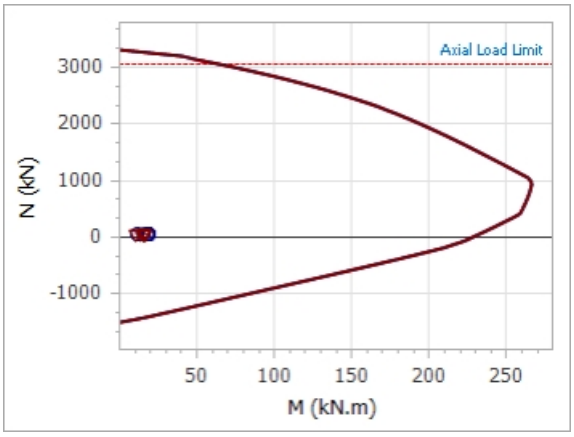


$V_{Ed(1/2)}$ =	11.6 / 10.6 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.08 / 0.08 N/mm²	λ_1/Lim_1 =	15.4 < 150.7 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.82 / 0.82 N/mm²	λ_2/Lim_2 =	15.4 < 150.7 ✓		
$v_{Rd\ Max}$ =	4.29 / 4.29 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	
Links = R10-300					

6C5 (B-6) (400/400)

(C25/30 / Grade 460 (Type 2))

No	$N_{Top/Bot}$ (kN)	$M_{11\ Top/Bot}$ (kN.m)	$M_{22\ Top/Bot}$ (kN.m)
1	60.7, 60.7	15.8, -13.2	-18.2, 15.6
2	61.2, 61.2	15.8, -13.3	-18.2, 15.5
3	45.1, 45.1	10.2, -9.8	-11.7, 11.7
4	61.1, 61.1	15.8, -13.3	-18.9, 15.7
5	60.4, 60.4	15.8, -13.2	-17.5, 15.4
6	60.5, 60.5	15.3, -13.1	-18.3, 15.6
7	60.9, 60.9	16.2, -13.3	-18.1, 15.5

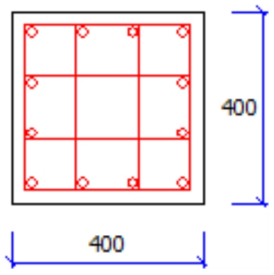


Critical Loading: 4 - (G+QP1 *F)

		min	Design	
N	69.3	-	69.3	kN
M_{11}	19.5	1.4	19.8	kN.m
M_{22}	-21.8	-1.4	-22.1	kN.m
N_{max}	2561.1			

Concrete Cover = 25.0 mm

Neutral Axis: 142.9 mm / 35.08 °



$V_{Ed(1/2)}$ =	13.3 / 12.3 kN	Short Column...		As (Req):	%0.20 (min) 320.00 mm²
v_{Ed} =	0.09 / 0.09 N/mm²	λ_1/Lim_1 =	15.4 < 247.8 ✓	As (Sup):	%2.36 3769.91 mm²
v_{Rdc} =	0.73 / 0.73 N/mm²	λ_2/Lim_2 =	15.4 < 249.6 ✓		
$v_{Rd\ Max}$ =	4.13 / 4.13 N/mm²	$M_{add(1/2)}$ =	0.0 / 0.0 kN.m	**12T20**	

Links = R10-300		

FOUNDATION

Project	PROPOSED 5-STOREY ADMINISTRATION	REINFORCED CONCRETE COUNCIL	
Client	ANSAH AND RITA AGYEMANG BARIMAH		
Location	Level -1 Base F1		
	PAD FOUNDATION DESIGN to BS 8110:1997 — Single column base		
	Originated from RCC81.xls on CD — © 1999 BCA for RCC		

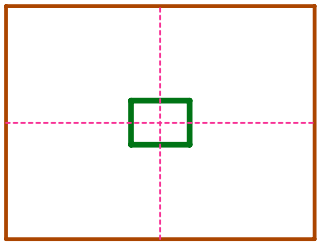
Made by	Date	Page
SAM	15/02/2022	
Checked	Revision	Job No
MICHAEL	-	

MATERIALS

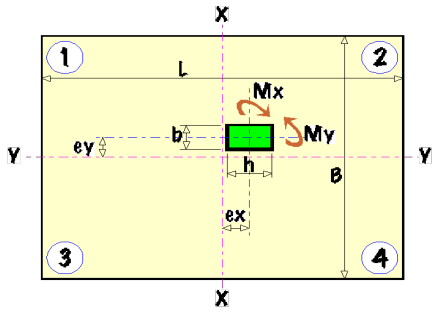
	fcu	25	N/mm²	h agg	19	mm	γc 1.5	concrete
	fy	460	N/mm²	cover	25	mm	γs 1.05	steel
Densities - Concrete		25	kN/m³	Soil	18	kN/m³		
Bearing pressure		490	kN/m² (net allowable increase)					

DIMENSIONS mm

BASE		COLUMN	
L =	2100	h =	400
B =	2100	b =	400
depth H =	400		
ex =	0	ey =	0



Plot (to scale)

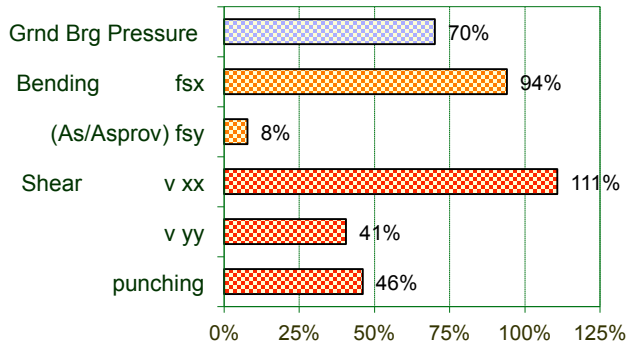


Key

COLUMN REACTIONS kN, kNm *characteristic*

	DEAD	IMPOSED	WIND
Axial (kN)	215.0	341.0	
Mx (kNm)	118.0	186.0	
My (kNm)			
Hx (kN)			
Hy (kN)			

STATUS VALID DESIGN



Efficiency

BEARING PRESSURES kN/m² *characteristic*

CORNER	1	2	3	4
no wind	0.0	343.4	0.0	343.4
with wind	0.0	343.4	0.0	343.4

REINFORCEMENT

Detail to 3.11.3.2

Mxx =	259.2	kNm	Myy =	145.6	kNm
b =	2100	mm	b =	2100	mm
d =	355	mm	d =	329	mm
As =	1754	mm²	As =	1064	mm²
PROVIDE	18 T40 @ 125 B1		PROVIDE	10 T12 @ 200 & 200 B2	
As prov =	22619	mm²	As prov =	1131	mm²

Asx increased 1,175% for shear

BEAM SHEAR

Vxx =	432.0	kN at d from col face	Vyy =	203.6	kN at d from col face
v =	0.579	N/mm²	v =	0.295	N/mm²
or Vxx =	775.3	kN at 2d from col face	or Vyy =	64.5	kN at 2d from col face
v =	1.040	N/mm²	v =	0.093	N/mm²
vc =	0.939	N/mm²	vc =	0.363	N/mm²

PUNCHING SHEAR

d ave =	342	mm	u crit =	4200	mm
As prov =	1.599	%	v max =	1.621	N/mm² at col face
v =	0.354	N/mm²	vc =	0.769	N/mm²

Project	PROPOSED 5-STOREY ADMINISTRATION			
Location	Level -1 Base F1		Made by SAM	Job No
	PAD FOUNDATION DESIGN to BS 8110:1997	Single column base	Date 8-Jan-99	
	Originated from RCC81.xls on CD		sheet 1 of 3	

REINFORCED CONCRETE COUNCIL

CALCULATIONS - SINGLE BASE

Gross base wt 44.10 L= 2.1 B= 2.1

CHARACTERISTIC PRESSURES

		G + Q	G + Q + W		G + W		
P	kN	600.10	600.10	OK	259.10		
Mx	kNm	304.00	304.00		118.00		
My	kNm	0.00	0.00		0.00		
ex	m	0.507	0.507		0.455	FS o'turn 2.072714	
ey	m	0.000	0.000		0.000	FS o'turn 100	0
6ex/L		1.4474	1.4474		1.3012		
if ex>L/6		2.5763	2.5763		2.3546		
k 2-4		2.5763	2.5763		2.3546		
k 1-3		0.0000	0.0000		0.0000		
6ey/B		0.0000	0.0000		0.0000		
if ey>B/6		1.3333	1.3333		1.3333		
k 1-2		1.0000	1.0000		1.0000		
k 3-4		1.0000	1.0000		1.0000		
P/A	kN/m²	136.08	136.08		58.75		
corner 1	kN/m²	0.00	0.00		0.00	Gross	
corner 2	kN/m²	350.57	350.57		138.34	Gross	
corner 3	kN/m²	0.00	0.00		0.00	Gross	
corner 4	kN/m²	350.57	350.57		138.34	Gross	

Text Asx increased
Asy increased

F/S OVERTURNING <1.5
OVERTURNS at ULTIMATE

Rebar ares

Area	Dia
50.3	8
78.5	10
113.1	12
201.1	16
314.2	20
490.9	25
804.2	32
1256.6	40

ULTIMATE PRESSURES

		1.4G+1.6Q	1.2(G+Q+W)		1.0G+1.4W		
N	kN	908.34	720.12	OK	276.74		
Mx	kNm	462.80	364.80		118.00		
My	kNm	0.00	0.00		0.00		
ex	m	0.510	0.507		0.426	FS o'turn 2.060841	
ey	m	0.000	0.000		0.000	FS o'turn 100	0
6ex/L		1.4557	1.4474		1.2183		
if ex>L/6		2.5902	2.5763		2.2450		
k 2-4		2.5902	2.5763		2.2450		
k 1-3		0.0000	0.0000		0.0000		
6ey/B		0.0000	0.0000		0.0000		
if ey>B/6		1.3333	1.3333		1.3333		
k 1-2		1.0000	1.0000		1.0000		
k 3-4		1.0000	1.0000		1.0000		
N/L	kN/m	432.54	342.91		131.78		
edge 2-4	kN/m	1120.37	883.45		295.85		
edge 1-3	kN/m	0.00	0.00		0.00		
N/B	kN/m	432.54	342.91		131.78		
edge 1-2	kN/m	432.54	342.91		131.78		
edge 3-4	kN/m	432.54	342.91		131.78		
3(L/2-ex)	m	1.621	1.630		1.871		
3(B/2-ey)	m	3.150	3.150		3.150		

XX MOMENTS

		1.4G+1.6Q		1.2(G+Q+W)		1.0G+1.4W		
		LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT	
from right	m	1.250	0.850					3.11.2.2
from left	m	0.850	1.250					3.11.2.2
to zero press	m	0.371	0.371	0.380	0.380	0.621	0.621	
local press	kN/m	256.69	0.00	206.06	0.00	98.18	0.00	
M1	kNm	30.9	269.8	24.8	212.8	11.8	71.3	
M2	kNm	25.5	0.0	20.9	0.0	17.1	0.0	
choose M	kNm	14.8	259.2	11.8	203.7	9.5	63.7	
Design Mxx	kNm	259.2						

XX SHEARS @ d from col face

from right	m	1.615	0.485					3.4.5.10
from left	m	0.485	1.615					3.4.5.8
to zero press	m	1.136	0.006	1.145	0.015	1.386	0.256	
local press	kN/m	785.26	4.49	620.62	8.27	219.15	40.46	
V1	kN	190.4	272.8	150.5	216.2	53.1	81.6	
V2	kN	446.2	2.6	355.4	4.7	151.9	28.0	
choose V	kN	432.0	258.5	343.2	204.0	141.7	71.4	
Design Vxx	kN	432.0						

@ 2d from col face

from right	m	1.980	0.120					3.4.5.10
from left	m	0.120	1.980					3.4.5.8
to zero press	m	1.501	-0.359	1.510	-0.350	1.751	-0.109	
local press	kN/m	1037.46	-247.71	818.42	-189.53	276.87	-17.27	
V1	kN	908.3	52.4	720.1	41.6	276.7	16.7	
V2	kN	778.9	-186.0	618.0	-143.1	242.4	-15.1	
choose V	kN	775.3	48.8	615.0	38.6	239.9	14.2	
Design Vxx	kN	775.3						

Project: PROPOSED 5-STOREY ADMINISTRATION
Location: Level -1 Base F1
PAD FOUNDATION DESIGN to BS 8110:1997 Single column base
Originated from RCC81.xls on CD
Made by SAM Job No
Date 8-Jan-99

REINFORCED CONCRETE COUNCIL

YY MOMENTS								
from top	m	1.250	0.850					3.11.2.2
from btm	m	0.850	1.250					3.11.2.2
to zero press	m	2.300	2.300	2.300	2.300	2.300	2.300	
local press	kN/m	432.54	432.54	342.91	342.91	131.78	131.78	
M1	kNm	156.3	156.3	123.9	123.9	47.6	47.6	
M2	kNm	381.4	381.4	302.3	302.3	116.2	116.2	
choose M	kNm	145.6	145.6	114.8	114.8	40.0	40.0	
Design Myy	kNm	145.6						
YY SHEARS	@ d from col face							
from top	m	1.595	0.505					3.4.5.10
from btm	m	0.505	1.595					3.4.5.8
to zero press	m	2.645	2.645	2.645	2.645	2.645	2.645	
local press	kN/m	432.54	432.54	342.91	342.91	131.78	131.78	
V1	kN	218.4	218.4	173.2	173.2	66.5	66.5	
V2	kN	572.0	572.0	453.5	453.5	174.3	174.3	
choose V	kN	203.6	203.6	160.4	160.4	55.9	55.9	
Design Vyy	kN	203.6						
	@ 2d from col face							
from top	m	1.940	0.160					3.4.5.10
from btm	m	0.160	1.940					3.4.5.8
to zero press	m	2.990	2.990	2.990	2.990	2.990	2.990	
local press	kN/m	432.54	432.54	342.91	342.91	131.78	131.78	
V1	kN	69.2	69.2	54.9	54.9	21.1	21.1	
V2	kN	646.7	646.7	512.7	512.7	197.0	197.0	
choose V	kN	64.5	64.5	50.8	50.8	17.7	17.7	
Design Vyy	kN	64.5						

REINFORCEMENT — min As 0.13%

		X-X		Y-Y						
b	mm	2100	choose dia	2100	choose dia					
dia	mm	40	20	12	20					
d	mm	355	365	329	345					
K'		0.1558		0.1558						
K		0.0392	0.0371	0.0256	0.0233					
z	mm	337.3	346.8	312.6	327.8	Punching				
As	mm²	1754	1706	1064	1014	ave ρ	0.00097	0.81252		
As shear	mm²	22365	22995	815	522	As X	930	0.48299		
required	mm²	22365	22995	1092	1092	As Y	522	1.29551		
No		18	7.09	10	6.85	ave ρ	0.00156	0.83541		
ave centres	mm	125	325	225	325	As X	1450	0.50648		
As prov	mm²	22619		1131		As Y	815	1.34188		
=	%	3.034		0.164						
Lc	mm	1050		1050						
Lc max	mm	1099	> 1,050	1040	< 1,050					
b central	mm	1465		1387						
Final centres	mm	125		200 & 200						

BEAM SHEAR

v @ d	N/mm²	0.5794	0.5636	0.2947	0.2810
v @ 2d	N/mm²	1.0400	1.0115	0.0934	0.0890
vc	N/mm²	0.9391	FAILS	0.3630	OK
As enhanced	%	########	1247.66%	0.00%	0.00%

PUNCHING SHEAR

d ave	mm	342	1.5 x d ave	513					355		
		L	R	B	T				532.5		
to edges	mm	1050	1050	1050	1050	1426					
to 1.5d per	mm	713	713	713	713	2100					
standard perimeter?	Y,N	0	0	0	0	0			732.5	732.5	
U	mm	0	0	0	0						732.5
within base?	Y,N	1	1	1	1	1	4200		1	1	1
		perimeter falls within base and on 2 sides									
U crit	mm	4200							5860		
Area	m²	2.995							2.146		
As prov	%	1.599									
vc	N/mm²	0.7685									

		1.4G+1.6Q		1.2(G+Q+W)		1.0G+1.4W				
N	kN	908.3		720.1		276.7				
v max	N/mm²	1.621	OK	1.285	OK	0.493	OK	2	1	0
ave press	kN/m²	133.54		106.18		43.06				
V	kN	508.4		402.2		147.8		621.7	492.2	184.3
v	N/mm²	0.3540	OK	0.2800	OK	0.1029	OK	0.2989	0.2366	0.0886

Project	PROPOSED 5-STOREY ADMINISTRATION		Made by	SAM
Location	Level -1 Base F1			Job No
	PAD FOUNDATION DESIGN to BS 8110:1997	Single column base	Date	8-Jan-99
	Originated from RCC81.xls on CD			sheet 3 of 3

REINFORCED CONCRETE COUNCIL

SIZING TIPS

Net press = 487.2

		G + Q	G + Q + W	G + W	
N	kN	556.0	556.0	215.0	
Mx	kNm	304.0	304.0	118.0	
My	kNm	0.0	0.0	0.0	
ex	m	547	547	549	
ey	m	0	0	0	
min A	m²	1.14	0.91	0.35	
min L	mm	400			
min B	mm	400			
sq base iteration 0		1.068	1.068	0.664	
		0	0	0	
		-1824	-1824	-708	
sq base iteration 1		2.137	2.137	1.329	
		3564	3564	857	
		1740	1740	149	
sq base iteration 2		1.693	1.693	1.235	
		1424	1424	651	
		-400	-400	-57	
sq base iteration 3		1.830	1.830	1.274	
		1969	1969	733	
		145	145	25	
SQUARE SIZE					
L & B	mm	1785	1785	1257	**1785 SQ**
if L =	2100				
		678	678	263	
B	mm	1392	1114	432	**or 2100 x 1395**
if B =	2100				
		265	265	102	
L	mm	1634	1431	833	**or 1635 x 2100**